SEISMIC SYSTEM/LOADING DATA:

SEISMIC SYSTEM/LOADING DATA:

NEW ADDITION: 2 STOREY ADDITION

SEISMIC FORCE RESISTING SYSTEM (SFRS)

SFRS: SYSTEM & CONNECTIONS: (2012 OBC CLAUSE 4.1.8.9/4.1.8.10)
LATERAL LOAD RESISTING SYSTEM: CONVENTIONAL CONSTRUCTION (REINFORCED MASONRY SHEARWALLS)
Rd = 1.5
Ro = 1.5
CSA STANDARD: CAN/CSA S304.1-04
APPLICABLE CLAUSE(S): 4.6.3

SFRS: DIAPHRAGMS & CONNECTIONS; (2012 OBC CLAUSE 4.1.8.15)
CSA STANDARD: CAN/CSA S16-09
APPLICABLE CLAUSE(S): 27.11.1 (b)

SFRS: SYSTEM FOUNDATIONS: (2012 OBC CLAUSE 4.1.8.16)
CSA STANDARD: CAN/CSA A23.3-04
APPLICABLE CLAUSE(S): 21.11 ☒ FOR ANCHORED FOOTINGS ☐ FOR UNANCHORED FOOTINGS

CONFIRMATION: FOUNDATIONS HAVE BEEN DESIGNED TO RESIST THE LATERAL LOAD CAPACITY OF THE SFRS INCLUDING ALL APPLICABLE AMPLIFICATION FACTORS

SEISMIC IMPORTANCE FACTOR; (2012 OBC CLAUSE 4.1.8.5)
Ie = 1.0

PROJECT CITY; OTTAWA (CITY HALL)

SITE CLASS; THE NOTED SITE CLASSIFICATION FOR SEISMIC SITE RESPONSE AND SHEAR WAVE VELOCITY PARAMETERS INDICATED ARE AS REPORTED IN THE GEOTECHNICAL REPORT # 1525834-1000 & 1525834-01 BY GOLDER ASSOCIATES.
REFER TO THE NOTED GEOTECHNICAL REPORT FOR V's, N60, AND/OR Su VALUES USED TO DETERMINE SITE CLASSIFICATION.
☐ A ☐ B ☒ C ☐ D ☐ E ☐ F (SITE SPECIFIC SPECTRUM: OTTAWA, CITY HALL)

PGA; 0.32

RESPONSE SPECTRUM DATA:

5% DAMPED SPECTRAL RESPONSE ACCELERATION VALUES; (2012 OBC SUPPLEMENT STANDARD SB-1)

Sa (0.2) = 0.640
Sa (0.5) = 0.310
Sa (1.0) = 0.140
Sa (2.0) = 0.046

DESIGN SPECTRAL RESPONSE ACCELERATION VALUES (DSRAV); (2012 OBC CLAUSE 4.1.8.4)

☒ CLASS C: (Fa=1.0/Fv=1.0)

S (0) = 0.64
S (0.2) = 0.64
S (0.5) = 0.31
S (1.0) = 0.14
S (2.0) = 0.046
S (4.0) = 0.023

SYSTEM RESTRICTION VALUE; IeFaSa(0.2) = 0.64 ≥ 0.35 ☒ YES ☐ NO

PERIOD DATA:

STATIC PERIOD; (2012 OBC CLAUSE 4.1.8.11(3))

Ta (STATIC) NS = 0.12 sec
Ta (STATIC) EW = 0.12 sec

DESIGN PERIODS/MODE & MOMENT FACTORS; (2012 OBC CLAUSE (4.1.8.11(5))

Sa(0.2) / Sa(2.0) = 13.91 ≥ 8.0 ☒ YES ☐ NO

Ta (DESIGN) NS = 0.18 sec MV = 1.0 J = 1.0
Ta (DESIGN) EW = 0.18 sec MV = 1.0 J = 1.0

DESIGN FUNDEMENTAL PERIOD BASED DSRAV:

S(Ta) NS = 0.64
S(Ta) EW = 0.64

IRREGULARITY REVIEW (2012 OBC CLAUSE 4.1.8.6)

1.	VERTICAL STIFFNESS:	☐ YES	☒ NO
2.	WEIGHT:	☐ YES	☒ NO
3.	VERTICAL GEOMETRIC:	☐ YES	☒ NO
4.	IN PLANE DISCONTINUITY:	☐ YES	☒ NO
5.	OUT OF PLANE:	☐ YES	☒ NO
6.	WEAK STOREY:	☐ YES	☒ NO
7.	TORSIONAL:	☐ YES	☒ NO
	B NS > 1.55		
	B EW > 1.36		
8.	NON-ORTHAGONAL:	☐ YES	☒ NO

CONCLUSION: BUILDING IS ☒ REGULAR ☐ IRREGULAR
DYNAMIC ANALYSIS: ☐ REQUIRED ☒ NOT REQUIRED
DYNAMIC PROCEDURE METHOD: ☐ MODAL RESPONSE SPECTRUM ☐ NUMERICAL INTEGRATION TIME HISTORY ☒ N/A

TORSIONAL ECCENTRICITY: ☒ ± 0.10 Dnx (4.1.8.11(10a), B ≤ 1.7 EQUIV. STATIC FORCE PROCEDURE)
☐ ± 0.10 Dnx (4.1.8.12(4a), B ≥ 1.7)
☐ ± 0.05 Dnx (4.1.8.12(4b), B < 1.7, 3-D DYNAMIC ANALYSIS)

STRUCTURAL SEPARATION: ☒ THE NEW AND EXISTING STRUCTURES HAVE BEEN SEPARATED IN ACCORDANCE WITH 4.1.8.14(1) OF THE 2012 O.B.C.
☐ N/A

BASE SHEARS/MOMENTS; (2012 OBC CLAUSE 4.1.8.11)
Vstatic = S(Ta)MvIeW/(RdRo) = 531 kN W = 1865 kN

STATIC MAXIMUM/MINIMUM VALUES:

NORTH-SOUTH; (↕)

Vmin = S(2.0)MvIeW/(RdRo) = 38 kN W = 1865 kN
Vmax = 2/3 S(0.2)IeW/(RdRo) = 355 kN W = 1865 kN

EAST-WEST; (↔)

Vmin = S(2.0)MvIeW/(RdRo) = 38 kN W = 1865 kN
Vmax = 2/3 S(0.2)IeW/(RdRo) = 355 kN W = 1865 kN

SEISMIC LOADS	
STATIC LOADS	DESIGN LOADS
NORTH-SOUTH; (↕)	
Vstns = 355 kN W = 1865 kN Mstns = 1953 kNm	Vdns = 355 kN Mdns = 1953 kNm
FAST-WEST; (↔)	
Vstew = 355 kN W = 1865 kN Mstew = 1953 kNm	Vdew = 355 kN Mdew = 1953 kNm

NOTES:

1) DYNAMIC LOAD SCALING FACTOR

$$S.F. = g \cdot \frac{Ie}{RdRo} = 0.444\ g$$

2) DESIGN LOAD SHEAR VALUES ARE BASED ON THE EVALUATION OF Vst AND Vd IN ACCORDANCE WITH 4.1.8.12 (5),(6),(7),(8), AND (9) OF THE 2012 OBC. LOADS INDICATED SHOW THE DESIGN BASE SHEAR AND CORRESPONDING OVERTURNING MOMENT.

WIND UPLIFT

(REF FIG I-9 NBC 2010 STRUCTURAL COMMENTARY I)
PNET = 1.4 (pe-pi) - 0.9 D
Pe = Iw q Ce Cp Cg
Pi = Iw q Ce Cpi Cgi
z = 1.0 m
Pf = 1.4 Pw NET - 0.9 Pd
Pw NET = Pe - Pi
Pw NET INTERIOR = 1.00 kPa
Pw NET PERIMETER = 1.14 kPa

DESIGN SNOW LOAD PARAMETERS
OTTAWA (CITY HALL), ONTARIO, CANADA
S= Is [Ss(CbCwCsCa)+Sr]
Ss = 2.4 kPa
Sr = 0.4 kPa
Is = 1.00
S= 1.00 [2.4(0.8x1.0x1.0x1.0)+0.4]
S= 2.32 kPa

WIND P= Iw q Ce Cg Cp
q = 0.41 kPa
Iw (uls) = 1.00 Iw (sls) = 0.75
Ce = VARIES FROM 0.9 TO 1.0
CpCg = 1.3 OR 1.95

	N.S (↕)	E.W (↔)	UNITS
VBASE	93	31	KN
MBASE	512	171	KN.m

GENERAL NOTES

1. ANY DEVIATION FROM THE CONDITIONS SHOWN ON THESE DRAWINGS MUST BE REPORTED TO THE ENGINEER.
2. THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF PART 4 OF THE O.B.C. (2012 EDITION) ONTARIO REGULATION 332/12 (AS AMENDED)
3. STANDARDS
   -CSA STANDARD A23.3-04 DESIGN OF CONCRETE STRUCTURES
   -CAN/CSA-S16-09 LIMIT STATES DESIGNS OF STEEL STRUCTURES
   -CSA STANDARD S304.1-04 DESIGN OF MASONRY STRUCTURES
   -CAN/CSA-086-09 ENGINEERING DESIGN IN WOOD
4. ANY MODIFICATIONS TO EXISTING STRUCTURES ARE TO BE LIMITED TO WORK NOTED ON THESE DRAWINGS. ANY ADDITIONAL OR PROPOSED MODIFICATIONS TO EXISTING STRUCTURES MUST BE APPROVED BY THE ENGINEER
5. FOUNDATIONS
   .1 ALL FOUNDATIONS ARE TO BEAR ON SOUND ROCK OR LEAN MIX MASS CONCRETE ON ROCK OR EXISTING FOOTING ON ROCK.
   .2 UNLESS NOTED OTHERWISE, THE BEARING CAPACITY USED IN THE FOUNDATION DESIGN IS ASSUMED TO BE: SLS= 500 kPa & ULS = 1000 kPa
   .3 BEARING SURFACE IS TO BE INSPECTED BY GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE.
   .4 FOR FURTHER INFORMATION SEE GEOTECHNICAL REPORT # 1525834-1000 & 1525834-01 BY GOLDER ASSOCIATES.
6. SLABS ON GRADE
   .1 SLABS ON GRADE TO BE UNREINFORCED UNLESS NOTED.
   .2 FOR COMPOSITION & COMPACTION OF FILL SUPPORTING SLABS ON GRADE SEE GEOTECHNICAL REPORT.
   .3 PROVIDE 12 mm ASPHALT IMPREGNATED FIBREBOARD BETWEEN SLABS ON GRADE & FOUNDATION WALLS OR COLUMNS
   .4 SAWCUT SLAB ON GRADE TO (1/4 x SLAB DEPTH) 8 HOURS AFTER CONCRETE PLACEMENT.
   .5 SPACE SAWCUTS ON A 4500 mm x 4500 mm MAXIMUM GRID. AVOID LONG & NARROW SAWCUT PATTERNS. LOCATE SAWCUTS ALONG COLUMN LINES WHERE POSSIBLE. CONTRACTOR IS TO PROVIDE THE ENGINEER WITH DOCUMENTATION SHOWING PROPOSED SAWCUT LOCATIONS FOR APPROVAL UNLESS SAWCUTS LOCATIONS ARE OTHEREWISE INDICATED ON THESE DRAWINGS.
7. MATERIALS
   .1 CONCRETE STRENGTH AT 28 DAYS TO BE AS NOTED ON PLANS AND IN SPECIFICATIONS
   .2 REINFORCING STEEL TO BE DEFORMED GRADE 400R WITH Fy= 400 MPa.
   .3 HOLLOW STRUCTURAL STEEL SECTIONS TO BE ASTM A500 GRADE C OR G40.21 350W CLASS C.
   .4 ALL 'W' SHAPE STEEL SECTIONS TO BE GRADE G40.21 350W WITH Fy= 350 MPa.
   .5 ALL OTHER STRUCTURAL STEEL TO BE GRADE G40.21 300W WITH Fy= 300 MPa UNLESS NOTED OTHERWISE.
   .6 ALL STRUCTURAL STEEL TO RECEIVE 1 SHOP APPLIED COAT OF PRIMER UNLESS NOTED.
   .7 ALL STRUCTURAL STEEL EXPOSED TO EXTERIOR IS TO BE HOT DIP GALVANIZED UNLESS NOTED.
   .8 ANCHOR BOLTS TO BE A307.
   .9 ALL OTHER BOLTS TO BE A325.
   .10 A325 BOLTS EXPOSED TO EXTERIOR ARE TO BE STAINLESS STEEL
   .11 A307 BOLTS EXPOSED TO EXTERIOR ARE TO BE GALVANIZED.
   .12 CONCRETE BLOCK TO BE H/15/A/M
   .13 CONCRETE BLOCK MASONRY MORTAR TO BE 8.5 MPa TYPE 'S' U/N.
   .14 CONCRETE BLOCK MASONRY GROUT TO BE 12 MPa "HIGH SLUMP" (200-250 mm SLUMP)
8. REINFORCING STEEL DESIGNATION

   8-20M x 1500 T/B

   8 = NUMBER OF BARS
   20M = SIZE OF BARS
   1500 = LENGTH OF BARS
   T = BAR LOCATION- TOP
   B = BAR LOCATION- BOT
   LENGTH OF BARS DOES NOT INCLUDE HOOKS OR BENDS
9. CONCRETE COVER
   .1 GRADE BEAMS 75 mm BOTTOM, 50 mm TOP & SIDES
   .2 FOUNDATION WALLS 40 mm UNLESS NOTED OTHERWISE
10. DOWELS
    DOWELS TO GRADE BEAMS TO BE OF SAME DIAMETER AS THE LOWEST LIFT OF VERTICAL REINFORCING IN PIERS OR WALLS.
11. REINFORCING STEEL SPLICES
    REINFORCING STEEL SPLICES TO BE AS NOTED IN REINFORCING BAR LAP LENGTH TABLE ON S01 U/N.
12. LOADS
    ALL LOADS & FORCES INDICATED ON THESE DRAWINGS ARE UNFACTORED WORKING LOADS UNLESS NOTED.
13. CONCRETE BLOCK MASONRY UNLESS NOTED OTHERWISE
    .1 140 mm CONCRETE BLOCK:
    VERT: 1-15M @ 800 o/c
    HORIZ: SL2 @ 200 o/c OR HL2 @ 400 o/c
    .2 190 mm CONCRETE BLOCK
    VERT: 1-15M @ 800 o/c
    HORIZ: HL2 @ 200 o/c
    .3 240 mm CONCRETE BLOCK
    VERT: 1-20M @ 800 o/c
    HORIZ: HL2 @ 200 o/c

    LEGEND
    H- HEAVY 5 mm LONGITUDINAL WIRES, 9 GAUGE CROSS WIRES
    L- LADDER TYPE REINFORCEMENT
    2- 2 LONGITUDINAL WIRES
    .5 SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR TYING MASONRY TO BACK UP WALLS.
    .6 SPECIAL WALLS - SEE NOTES ON PLANS & SCHEDULE FOR REINFORCING AND GROUTING OTHER THAN INDICATED ABOVE
    .7 REINFORCE CELLS @ END OF WALLS AT INTERSECTING WALLS & BESIDE OPENINGS.
    .8 GROUT MASONRY SOLID BELOW BEARING BASE PLATES FOR 500mm MIN.
    .9 PROVIDE "CLEAN OUTS" AT BOTTOM OF CELLS TO BE GROUTED TO ENSURE PROPER LAP LENGTH AND THAT CELL IS FILLED SOLIDLY. MAXIMUM GROUT LIFT IS 3 metres. GROUT TO HAVE 250mm SLUMP.
    .10 EMBEDMENT OF MASONRY DOWELS IN CONCRETE STRUCTURE
    BELOW CONCRETE BLOCK WALLS TO BE AS FOLLOWS:
    15M DOWELS = 600 mm EMBEDMENT - 1300 Lg. DOWEL
    20M DOWELS = 800 mm EMBEDMENT - 1700 Lg. DOWEL
    .11 BLOCK CONTROL JOINT SPACED AT 9000 mm MAXIMUM VENEER CONTROL JOINT SPACED AT 12000 mm MAXIMUM COORDINATE LOCATION OF JOINTS WITH ARCHITECT & ENGINEER
14. LEGEND

B	=	BOTTOM
B1	=	BOTTOM LOWER LAYER
B2	=	BOTTOM UPPER LAYER
BLL	=	BOTTOM LOWER LAYER
BBP1	=	BEAM BEARING PLATE NUMBER
BP1	=	BASE PLATE NUMBER
BUL	=	BOTTOM UPPER LAYER
DP	=	DEPTH
EF	=	EACH FACE
EL	=	ELEVATION
ES	=	EACH SIDE
EW	=	EACH WAY
(ex)	=	EXISTING
GB	=	GRADE BEAM
H	=	HORIZONTAL
(H)	=	HOOKED BAR
O/C	=	ON CENTER
SC1	=	STEEL COLUMN NUMBER
T	=	TOP
T1	=	TOP UPPER LAYER
T2	=	TOP LOWER LAYER
TLL	=	TOP LOWER LAYER
TUL	=	TOP UPPER LAYER
U/N	=	UNLESS NOTED OTHERWISE
V	=	VERTICAL

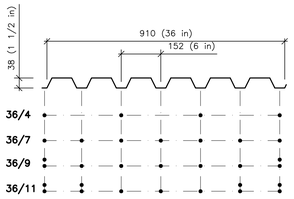
DESIGN & DETAILING CRITERIA FOR SUPPLIERS

1. MISCELLANEOUS METALS & STEEL STAIRS
   MISC METALS & STEEL STAIRS ARE TO BE DESIGNED AND DETAILED BY MISC METALS & STEEL STAIRS SUPPLIER. SHOP DRAWINGS ARE TO BE SUBMITTED TO DESIGN TEAM FOR REVIEW. SHOP DRAWINGS ARE TO BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO. ALL MISC METAL & STEEL STAIR WORK IS TO BE INSPECTED DURING CONSTRUCTION BY THE MISC METALS & STEEL STAIRS DESIGN ENGINEER.
2. COLD FORMED STEEL STUDS & JOISTS
   STEEL STUDS & JOISTS ARE TO BE DESIGNED AND DETAILED BY STEEL STUDS & JOISTS SUPPLIER. SHOP DRAWINGS ARE TO BE SUBMITTED TO DESIGN TEAM FOR REVIEW. SHOP DRAWINGS ARE TO BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO. ALL STEEL STUD & JOIST WORK IS TO BE INSPECTED DURING CONSTRUCTION BY THE STEEL STUD & JOIST DESIGN ENGINEER.
3. STRUCTURAL STEEL CONNECTIONS
   STRUCTURAL STEEL CONNECTIONS ARE TO BE DESIGNED AND DETAILED BY STRUCTURAL STEEL SUPPLIER. SHOP DRAWINGS ARE TO BE SUBMITTED TO CUNLIFFE & ASSOCIATES FOR REVIEW. SHOP DRAWINGS ARE TO BE STAMPED AND SIGNED "FOR CONNECTIONS ONLY" BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO. INSPECTION OF WELDS, CONNECTIONS & INSTALLATION IS TO BE UNDERTAKEN BY A 3RD PARTY, CERTIFIED INSPECTION SERVICE.
4. GUARDS & HANDRAILS
   GUARDS & HANDRAILS ARE TO BE DESIGNED AND DETAILED BY STEEL SUPPLIER. SHOP DRAWINGS ARE TO BE SUBMITTED TO DESIGN TEAM FOR REVIEW. SHOP DRAWINGS ARE TO BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO. ALL GUARDS & HANDRAIL WORK IS TO BE INSPECTED DURING CONSTRUCTION BY THE GUARD & HANDRAIL DESIGN ENGINEER.
5. SEISMIC RESTRAINT OF MECH'L EQUIPMENT & PIPING
   SEISMIC RESTRAINT OF MECH'L EQUIPMENT & PIPING TO BE DETAILED BY MECH'L EQUIPMENT & PIPING SUPPLIER OR CONTRACTOR. SHOP DRAWINGS ARE TO BE SUBMITTED TO CUNLIFFE & ASSOCIATES FOR REVIEW. SHOP DRAWINGS ARE TO BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO. ALL SEISMIC RESTRAINT INSTALLATIONS ARE TO BE INSPECTED DURING CONSTRUCTION BY THE DESIGN ENGINEER OF RECORD
6. CONCRETE BLOCK MASONRY WALLS-CONSTRUCTION BRACING
   ALL NON LOAD BEARING CONCRETE BLOCK MASONRY WALLS ARE TO BE LATERALLY BRACED DURING CONSTRUCTION UNTIL PERMANENT LATERAL BRACING IS INSTALLED AS PER TYPICAL DETAILS AND OR SECTIONS.

   LATERAL CONSTRUCTION BRACING DRAWINGS ARE TO BE SUBMITTED TO CUNLIFFE & ASSOCIATES FOR REVIEW. SHOP DRAWINGS ARE TO BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO.
7. CURTAIN WALLS
   SUPPORTS FOR CURTAIN WALLS ARE TO BE DESIGNED AND DETAILED BY CURTAIN WALL SUPPLIER. SHOP DRAWINGS ARE TO BE SUBMITTED TO DESIGN TEAM FOR REVIEW. SHOP DRAWINGS ARE TO BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO. ALL SUPPORTS ARE TO BE INSPECTED DURING CONSTRUCTION BY THE SUPPORT DESIGN ENGINEER.

NOTE:
INSPECTION REPORTS CREATED AS A RESULT OF THE ABOVE NOTED WORK MUST BE SUBMITTED TO THE CONSTRUCTION MANAGER. CONSTRUCTION MANAGER IS TO PROVIDE COPIES TO THE CONSULTANTS.

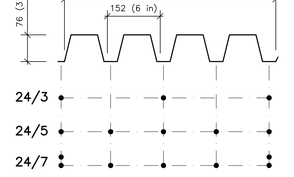
DRAWING LIST



FASTENER PATTERNS @ SUPPORT FOR 38 THK STEEL DECK

STEEL DECK NOTES: TYPICAL FLOOR DECK (SEE PLAN ALSO)
1. 38 x 0.76 COMPOSITE INTERLOCKING (CANAM OR EQUIVILENT)
2. HILTI S-SLC01M HWH FASTENERS IN SIDE LAPS @ 300 o/c
3. 36/7 FASTENER PATTERN (SEE ABOVE)
4. HILTI X-HSN24 FASTENERS TO SUPPORTING MEMBERS
5. FASTENER SPACING AROUND PERIMETER & OPENINGS TO BE 150 o/c
6. DECK BE 3 SPAN MINIMUM
7. STEEL DECK IS NOT TO BE USED FOR SUPPORT OF ARCH'L, MECH'L OR ELECT'L ITEMS. USE STEEL STRUCTURE FOR SUPPORT.

STEEL DECK NOTES: TYPICAL ROOF DECK (SEE PLAN ALSO)
1. 38 x 1.21 OVERLAPPING (CANAM OR EQUIVILENT)
2. HILTI S-SLC01M HWH FASTENERS IN SIDE LAPS @ 150 o/c
3. 36/11 FASTENER PATTERN (SEE ABOVE)
4. HILTI X-HSN24 FASTENERS TO SUPPORTING MEMBERS
5. FASTENER SPACING AROUND PERIMETER & OPENINGS TO BE 150 o/c U/N OTHERWISE (SEE PLAN).
6. DECK BE 3 SPAN MINIMUM
7. STEEL DECK IS NOT TO BE USED FOR SUPPORT OF ARCH'L, MECH'L OR ELECT'L ITEMS. USE STEEL STRUCTURE FOR SUPPORT.



FASTENER PATTERN @ SUPPORT FOR 76 THK ACOUSTIC STEEL DECK

STEEL DECK NOTES: ACITIVITY ROOM ROOF DECK (SEE PLAN ALSO)
1. 76 x 1.21 OVERLAPPING (CANAM OR EQUIVILENT)
2. HILTI S-SLC01M HWH FASTENERS IN SIDE LAPS @ 75 o/c
3. 24/7 FASTENER PATTERN (SEE ABOVE)
4. HILTI ENP-19 FASTENERS TO SUPPORTING MEMBERS
5. FASTENER SPACING AROUND PERIMETER & OPENINGS TO BE 150 o/c
6. DECK BE 3 SPAN MINIMUM
7. STEEL DECK IS NOT TO BE USED FOR SUPPORT OF ARCH'L, MECH'L OR ELECT'L ITEMS. USE STEEL STRUCTURE FOR SUPPORT.

REINFORCING BAR LAP LENGTH TABLE

CONCRETE STRENGTH (MPa)	REINFORCING BAR LAP LENGTH (mm)				
	10M	15M	20M	25M	30M
20	475	700	850	1325	1575
25	425	600	750	1200	1400
30	400	550	675	1100	1275
35	375	525	625	1000	1200

FOR SPECIAL CONDITIONS MULTIPLY THE VALUES LISTED ABOVE BY THE FOLLOWING FACTORS:
1. EPOXY COATED REINFORCING (X 1.5)
2. HORIZONTAL REINFORCING WITH >300 mm CONCRETE BELOW (X 1.3)
3. FOR CONDITIONS 1 & 2 OCCURING SIMULTANEOUSLY (X 1.7)

CONCRETE BLOCK MASONRY WALLS
REINFORCING BAR LAP LENGTH TABLE
REINFORCING BAR LAP LENGTH (mm)

HJR	10M	15M	20M	25M	30M
300	525	750	925	1450	1725

FOR SPECIAL CONDITIONS MULTIPLY THE VALUES LISTED ABOVE BY THE FOLLOWING FACTORS:
1. EPOXY COATED REINFORCING (X 1.5)
2. HORIZONTAL REINFORCING WITH >300 mm GROUT BELOW (X 1.3)
3. FOR CONDITIONS 1 & 2 OCCURING SIMULTANEOUSLY (X 1.7)

DEPARTMENT OF CORPORATE SERVICES / SERVICES GENERAUX
REAL PROPERTY ASSET MANAGEMENT BRANCH /
DIRECTION DE LA GESTION DES ACTIFS ET DES BIENS IMMOBILIERS
DESIGN AND CONSTRUCTION DIVISION /
DIVISION DE LA CONCEPTION ET DE LA CONSTRUCTION

GREG GEDDES CHIEF CORPORATE SERVICES OFFICER /
CHEF DES SERVICES GÉNÉRAUX
S.A. FINNAMORE, O.A.A. DIRECTOR / DIRECTEUR, O.A.O.
ROBERT VAILLANCOURT, O.A.A. MANAGER / GESTIONNAIRE, O.A.O.

FOR / POUR

Dovercourt RECREATION CENTRE

CUNLIFFE & ASSOCIATES
CONSULTING STRUCTURAL ENGINEERS
102-1737 WOODWARD DR. OTTAWA ON. K2C 0P9
TEL (613) 729-7242 FAX (613) 728-1461
Email <cunliffe@cunliffe.ca>

NUMBER/ NUMERO	MILESTONE / FAIT SAILLANT	DATE (Y/M/D) (AMJ)	INITIALS INITIALES
6			
5			
4			
3			
2			
1	ISSUED FOR INFORMATION	2017/06/23	RW

DESIGNED BY / CONCU PAR: JC
CHECKED BY / VERIFIE PAR: JC
DRAWN BY / DESSINE PAR: RW
SCALE / ECHELLE: NOT TO SCALE

DETAIL NUMBER 1 / A1.1 DRAWING TITLE
SCALE:
SHEET NUMBER

ARCHITECT / ARCHITECTE

CONSULTANT / EXPERT-CONSEIL

CONSULTANT / EXPERT-CONSEIL

CONSULTANT / EXPERT-CONSEIL

PROJECT / LOCATION / PROJET / ENDROIT

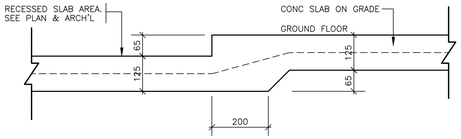
DOVERCOURT
PHASE 2 ADDITION & RENOVATION
411 DOVERCOURT
OTTAWA, ONTARIO

DRAWING / DESSIN
SEISMIC DATA & GENERAL NOTES

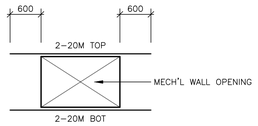
FACILITY NO. / NO. DE INSTALLTIONS

PROJECT NO. / PROJET NO.
1713

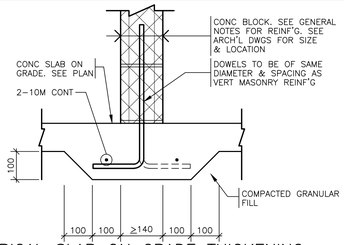
SHEET NO. / FEUILLE No.
S01



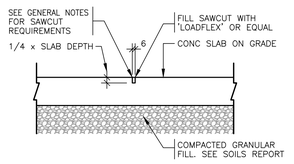
TYP. DETAIL– RECESSED SLAB ON GRADE



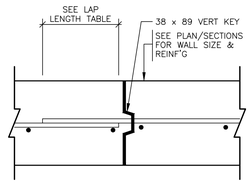
TYPICAL DETAIL AT CONCRETE WALL OPENING U/N



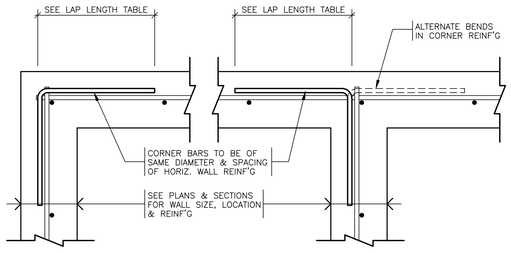
TYPICAL SLAB ON GRADE THICKENING
DETAIL BELOW NON LOAD BEARING
CONC BLOCK MASONRY



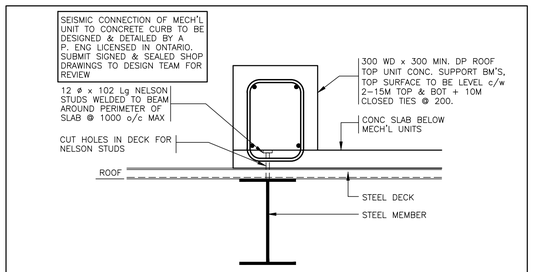
TYPICAL SAWCUT IN SLAB ON GRADE DETAIL



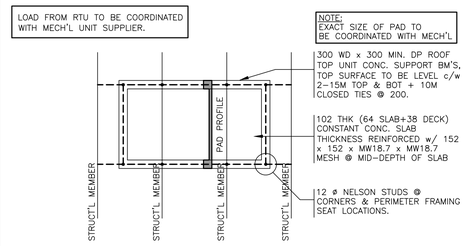
TYPICAL WALL CONSTRUCTION JOINT DETAIL
MAXIMUM SPACING OF CONSTRUCTION JOINTS TO BE 20 metres



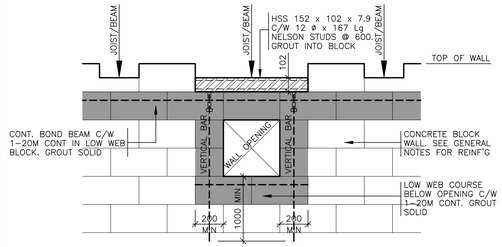
TYPICAL WALL INTERSECTION REINFORCEMENT
CONCRETE WALLS WITH 1 SHEET OF REINFORCING (WALL THICKNESS LESS THAN 215 mm)
NOT APPLICABLE TO SHEARWALLS. SEE SHEARWALL ELEVATION DRAWINGS



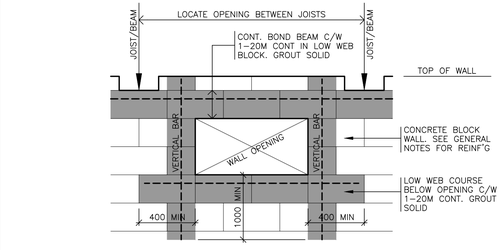
DETAIL–NELSON STUDS AT
PERIMETER OF MECH'L PAD



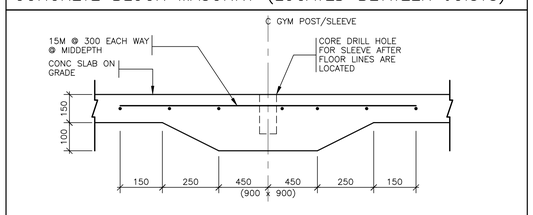
TYPICAL SCHEMATIC DETAIL– CONCRETE PAD
BELOW ROOFTOP MECH'L UNITS



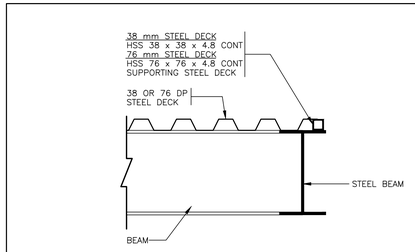
ELEVATION–MECHANICAL OPENING THRU LOAD BEARING
CONCRETE BLOCK MASONRY (LOCATED BELOW JOISTS)



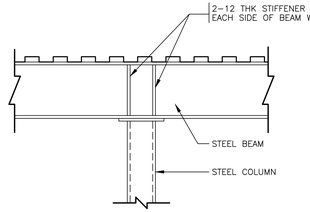
ELEVATION–MECHANICAL OPENING THRU LOAD BEARING
CONCRETE BLOCK MASONRY (LOCATED BETWEEN JOISTS)



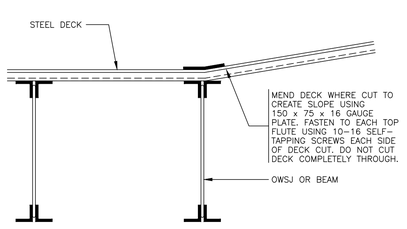
TYPICAL SLAB ON GRADE THICKENING DETAIL
AT GYMNASIUM FLOOR SOCKET
COORDINATE LOCATIONS WITH ARCH'L DRAWINGS



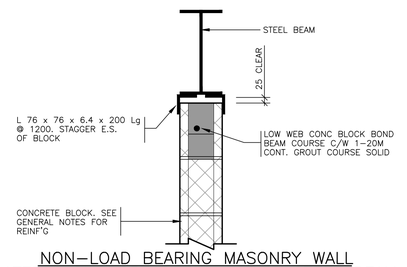
TYP. DETAIL–ALTERNATE SUPPORT OF STEEL DECK



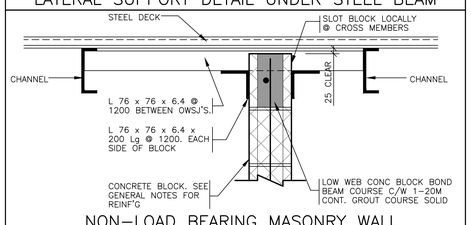
CANTILEVERED OR MULTI–SPAN BEAM DETAIL



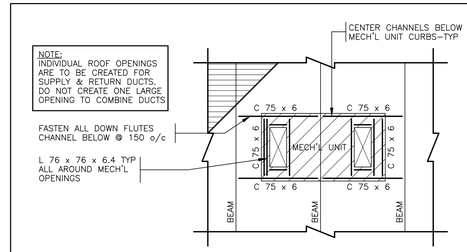
DETAIL–MENDING DECK AT CUT LINES



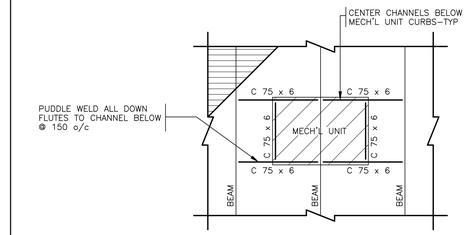
NON–LOAD BEARING MASONRY WALL
LATERAL SUPPORT DETAIL UNDER STEEL BEAM



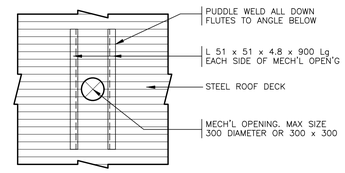
NON–LOAD BEARING MASONRY WALL
LATERAL SUPPORT DETAIL PARALLEL TO CHANNELS



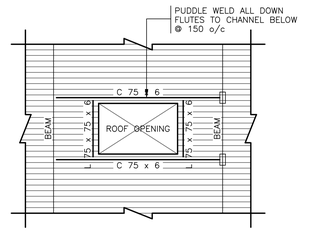
TYPICAL DETAIL– ADDITIONAL
FRAMING AT ROOFTOP MECH'L UNITS
(WITH UNIT OPENINGS THROUGH ROOF)



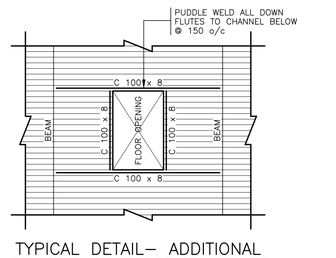
TYPICAL DETAIL– ADDITIONAL FRAMING
AT ROOFTOP MECH'L UNITS (U/N ON PLAN)
(NO UNIT OPENINGS THROUGH ROOF)



PLAN DETAIL–MECHANICAL OPENINGS



TYPICAL DETAIL– ADDITIONAL
FRAMING AT ROOF OPENINGS



TYPICAL DETAIL– ADDITIONAL
FRAMING AT FLOOR OPENINGS

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1	ISSUED FOR INFORMATION	2017/06/23	RW
NUMBER/ NUMÉRO	MILESTONE / FAIT SAILLANT	DATE (Y/M/D) (AA/M/J)	INITIALS INITIALES

DESIGNED BY / CONCU PAR: JC	CHECKED BY / VERIFIÉ PAR: JC
DRAWN BY / DESSINÉ PAR: RW	SCALE / ECHELLE: NOT TO SCALE

DETAIL NUMBER 1 / A1.1 SHEET NUMBER — DRAWING TITLE — SCALE:

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CONSULTANT / EXPERT-CONSEIL	CONSULTANT / EXPERT-CONSEIL

PROJECT / LOCATION / PROJET / ENDROIT

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ADDITION & RENOVATION
411 DOVERCOURT
OTTAWA, ONTARIO

DRAWING / DESSIN
TYPICAL DETAILS

FACILITY NO. / NO. DE INSTALLTIONS

PROJECT NO. / PROJET NO.
1713

SHEET NO. / FEUILLE No.
S02

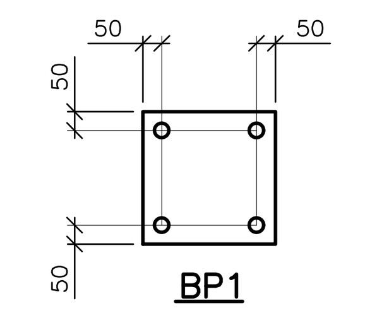
CAISSON SCHEDULE		
MARK	SIZE	REINF'G
CA1	600 DIAMETER	8–20M VERTICAL 1–10M TIE @ 300 o/c DRILL 200 DP HOLES INTO (ex) FOOTING TO ACCEPT 4–20M x 1000 Lg DWLS. SECURE IN HOLE USING HILTI HIT HY200 ADHESIVE. LAP DWLS WITH ALTERNATE VERTICAL REINFORCING. PROVIDE 90° STANDARD BEND AT TOP OF 8–20M VERTICAL & LOCATED BEND 50 mm BELOW TOP OF GRADE BEAM

STEEL COLUMN SCHEDULE	
MARK	SIZE
SC1	HSS 127 x 127 x 6.4

NOTES:

1. ALL HSS SECTIONS TO BE ASTM A500 (GRADE C) OR G40.21M350W (CLASS C).
2. SEE ALSO GENERAL NOTES ON S01.

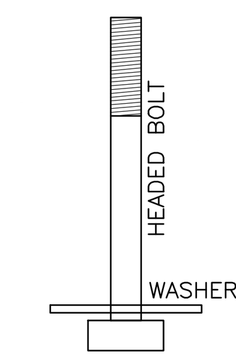
BASEPLATE SCHEDULE			
MARK	SIZE	ANCHORS	WASHERS
BP1	350 x 350 x 20 THK PLATE	4–M20 ø HEADED ANCHOR BOLTS 300 mm EMBEDMENT	50 ø x 5 THK



BP1

NOTE:

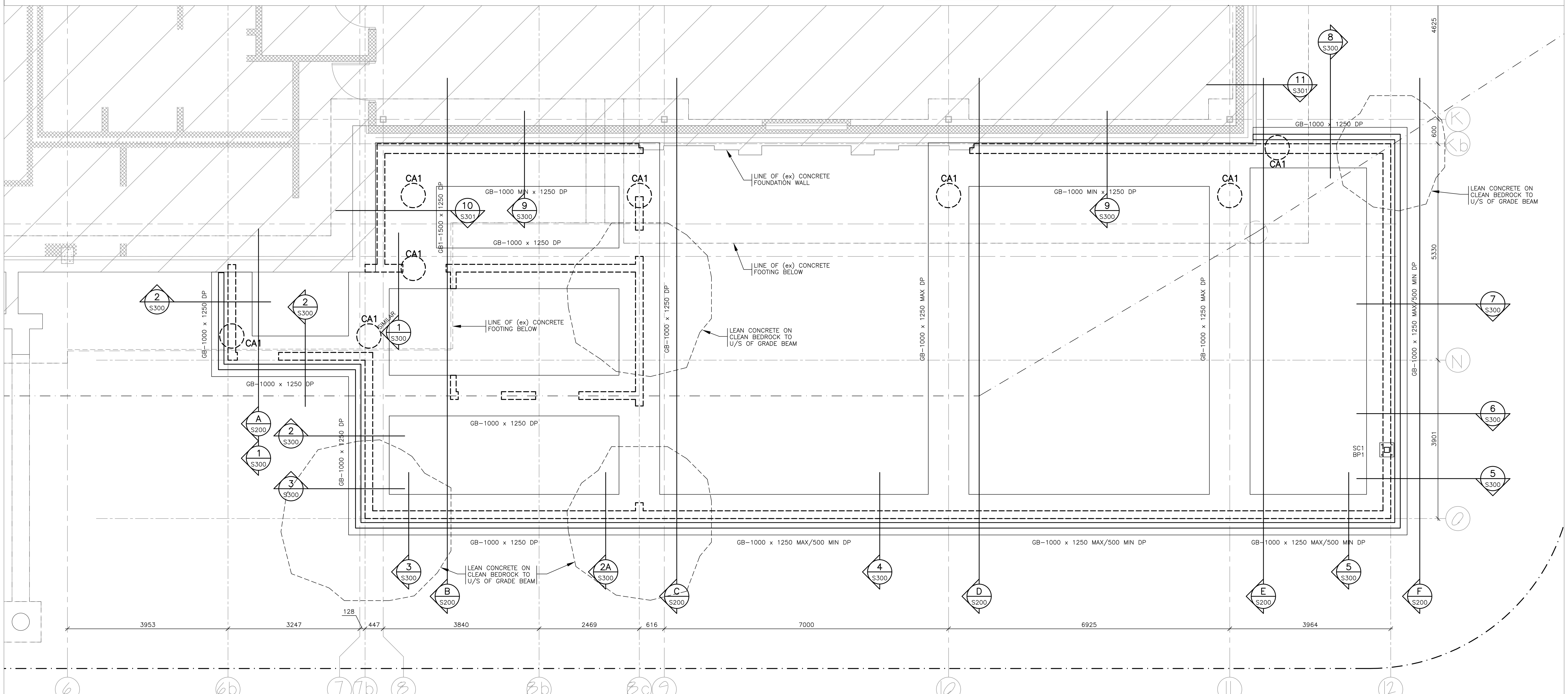
1. PROVIDE 25 mm NON SHRINK GROUT OR DRYPACK BELOW BASEPLATES EXTENDING 25 mm BEYOND PERIMETER OF PLATE AND FOR FULL AREA BELOW PLATE (NOT APPLICABLE TO CAST–IN PLATES)
2. PROVIDE 50 mm ANCHOR BOLT PROJECTION ABOVE PLATE.
3. ALL ANCHOR BOLTS TO BE ASTM A307 u/n
4. ALL BASE PLATES TO BE GRADE G40.21300W
5. ALL COLUMNS TO BE CENTRED ON BASEPLATES U/N
6. USE STEEL TEMPLATES AND PRECISE SURVEYING TEQHNIQUES TO ACCURATELY LOCATE BASE PLATE & ANCHOR BOLTS.



GRADE BEAM SCHEDULE		
MARK	SIZE	REINFORCING
GB	1000 x 1250 DP	4–25M T & B CONT 2–25M EF EQUALLY SPACED BTW T & B 1–15M CLOSED TIE @ 300 o/c
GB1	1500 x 1250 DP	5–25M T & B CONT 2–25M EF EQUALLY SPACED BTW T & B 1–15M CLOSED TIE @ 300 o/c

NOTES:

1. ENDS OF HORIZONTAL CONT BARS ARE TO TERMINATE WITH 90° x 300 Lg BENDS AT THE FAR FACE OF THE WALL OR END OF BEAM.
2. WHERE DEPTH OF GRADE BEAM VARIES, USE 15M "U" BARS T & B @ 300 o/c TO CREATE A CLOSED TIE
3. REINFORCING LAPS
   TOP REINF'G @ MID–SPAN BETWEEN PIERS
   LAP BOTTOM REINF'G @ PIERS
   LAP REMAINING REINF'G @ PIERS



FOUNDATION PLAN

SCALE 1 : 50

CONCRETE COMPRESSIVE STRENGTH

MASS CONCRETE FILL – 15 MPa TYPE N
INTERIOR SLAB ON GRADE – 25 MPa TYPE N
EXTERIOR SLAB ON GRADE – 32 MPa CLASS C2
PERIMETER FOUNDATION WALLS – 25 MPa CLASS F2
GRADE BEAMS – 25 MPa TYPE N
CAISSONS – 25 MPa TYPE N

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2			
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DRAWN BY / DESSINE PAR RW	SCALE / ECHELLE 1:50

DETAIL NUMBER
1 / A1.1 DRAWING TITLE
SCALE:
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ADDITION & RENOVATION

411 DOVERCOURT
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DRAWING / DESSIN

FLOOR PLANS
FOUNDATION PLAN

FACILITY NO. / NO. DE INSTALLTIONS

SHEET NO. / FEUILLE No.
S100

PROJECT NO. / PROJET NO.
1713

MASONRY VENEER LOOSE LINTEL SCHEDULE

SPAN	SIZE	BEARING EE
0 – 1200 mm	L 89 x 89 x 8	100 mm
1201 TO 1800	L 102 x 89 x 8 LLV	100 mm
1801 – 2400	L 127 x 89 x 8 LLV	100 mm
2401 – 3000	L 152 x 102 x 8 LLV	150 mm
3001 – 3400	L 152 x 102 x 9.5 LLV	175 mm

NOTE:
1. ALL "EXTERIOR" ANGLES ARE TO BE HOT DIP GALVANIZED

STEEL COLUMN SCHEDULE

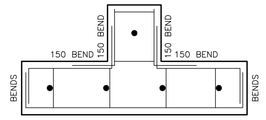
MARK	SIZE
SC1	HSS 127 x 127 x 6.4

NOTES:
1. ALL HSS SECTIONS TO BE ASTM A500 (GRADE C) OR G40.21M350W (CLASS C).
2. SEE ALSO GENERAL NOTES ON S01.

MASONRY PIER SCHEDULE

MARK	SIZE	REINFORCEMENT
MP1	790 Lg x THICKNESS OF BLOCK	4–20M VERT. + 4–20M DOWEL c/w 1 – 4.88 ø GALVANIZED WIRE TIE @ 200 o/c
MP2	990 x 190	5–20M VERT. + 5–20M DOWEL c/w 1 – 4.88 ø GALVANIZED WIRE TIE @ 200 o/c
MP3	190 x 800+ 190 x 200 (SEE PLAN)	5–20M VERT. + 5–20M DOWEL c/w HEAVY DUTY LADDER TIE @ 200 o/c (SEE DETAIL BELOW)

NOTES:
1. ALL MASONRY PIERS TO BE GROUTED SOLID.
2. ALL MASONRY PIERS TO BE FULL HEIGHT OF WALL



MP3

CONCRETE BLOCK WALL MASONRY LINTEL SCHEDULE UNLESS NOTED WITH ML#

140 OR 190 WD REINF'D CONC BLOCK

SPAN	SIZE & REINF'G
0 – 1000 mm	200 DP 1–15M BOT CONT
1001 – 1900	400 DP 1–20M BOT CONT
1901 – 2500	600 DP 1–20M TOP & BOT CONT
2501 – 3200	800 DP 1–20M TOP & BOT CONT

NOTE:
1. ALL MASONRY LINTELS ARE TO BE GROUTED SOLID
2. GROUT WALL ENDS SOLID BELOW LINTEL FOR WIDTH OF 200 mm LINTEL BEARING
3. SEE DRAWING S00 FOR MASONRY REINFORCEMENT. PROVIDE 1–20M VERT IN EACH GROUTED CORE AT WALL ENDS EACH SIDE OF LINTEL OPENING UNLESS NOTED OTHERWISE ON PLANS.
4. USE LOW WEB BLOCKS FOR LINTEL COURSES CONTAINING HORIZONTAL REBAR

LOAD BEARING MASONRY LINTEL SCHEDULE

MARK	SIZE	REINF'G	BEARING
ML1	190 x 590 DP	1–20M BOTTOM	200
ML2	190 x 390 DP	1–20M BOTTOM	200
ML3	190 x 990 DP	1–20M BOTTOM 1–15M TOP & MID	300

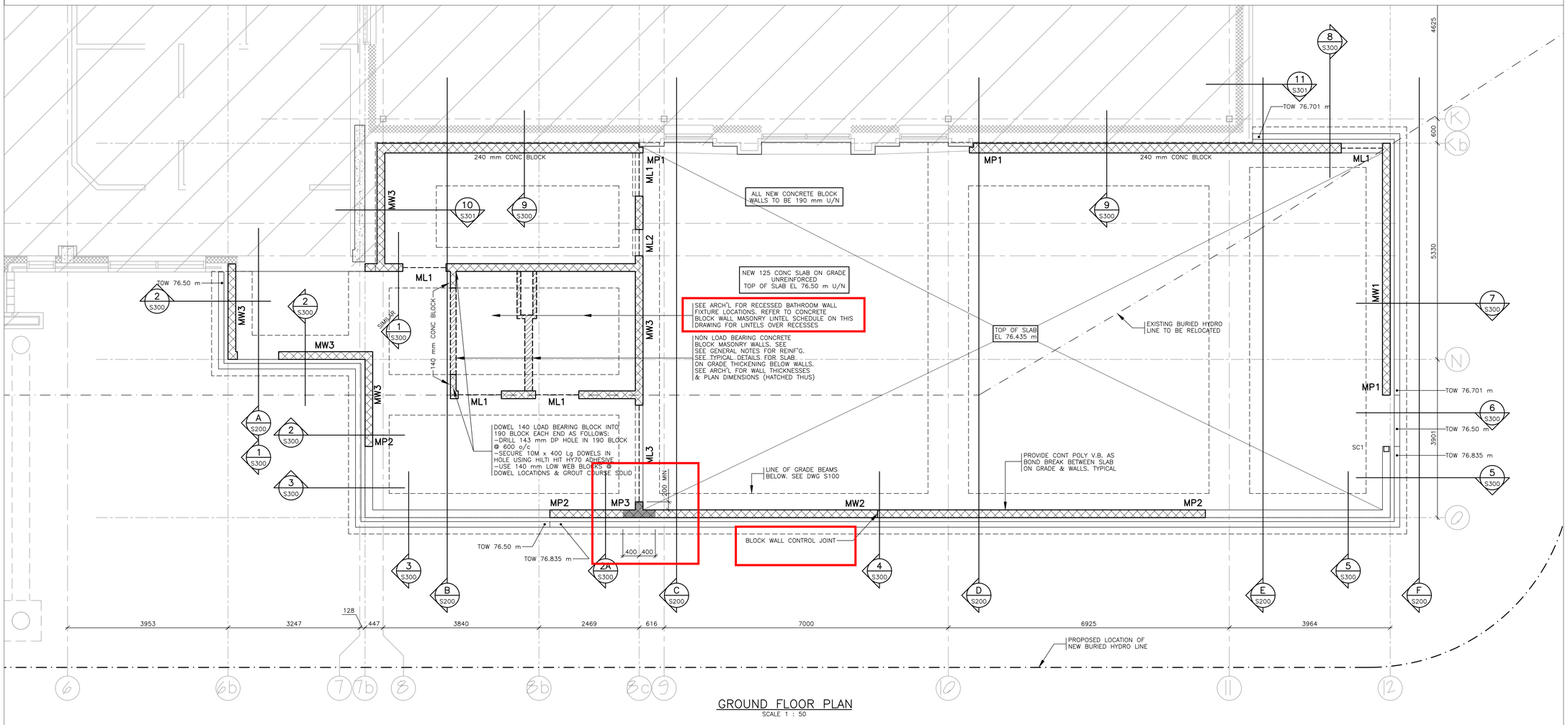
NOTE:
1. ALL LINTELS ARE TO BE GROUTED SOLID
2. GROUT WALL ENDS SOLID BELOW LINTEL FOR WIDTH OF BEARING INDICATED ABOVE.
3. SEE DRAWING S01 & SCHEDULE FOR MASONRY REINFORCEMENT. PROVIDE 1–20M VERT IN EACH GROUTED CORE AT WALL ENDS EACH SIDE OF LINTEL OPENING UNLESS NOTED OTHERWISE ON PLANS.
4. USE SPECIAL LINTEL BLOCKS FOR LOWEST LINTEL COURSE AND USE LOW WEB BLOCKS FOR ALL OTHER LINTEL COURSES.
5. DO NOT INTERRUPT TYPICAL WALL REINFORCEMENT AT LINTELS.

MASONRY WALL SCHEDULE

MARK	THICKNESS/ GROUTING	REINFORCING VERT.	REINFORCING HORIZ.
MW1	190mm GROUTED @ 600	15M @ 600 o/c	HL2 @ 200 o/c
MW2	190mm GROUTED @ 400	15M @ 400 o/c	HL2 @ 200 o/c
MW3	190mm GROUTED SOLID	15M @ 400 o/c	HL2 @ 200 o/c

NOTE:
1. PROVIDE ADDITIONAL REINFORCING AS FOLLOWS: 1–20M VERT. @ ES OPENINGS, CORNERS & INTERSECTIONS FULLY GROUTED
2. ALL BLOCK WALL SECTIONS EQUAL TO OR LESS THAN 800 mm IN LENGTH TO BE FULLY GROUTED AND REINFORCED WITH 15M VERTICAL @ 200 o/c & HL2 @ 200 o/c (UNLESS NOTED OTHERWISE).
3. SEE PLANS FOR LOCATIONS & EXTENT OF MW #.
4. SEE GENERAL NOTES ON DWG S01 FOR TYPICAL MASONRY WALL REINFORCING UNLESS NOTED OTHERWISE IN SCHEDULE OR ON PLAN.
5. SEE PLANS FOR LOCATIONS OF CONTROL JOINTS (NOTED CJ ON PLAN)

LEGEND
H– HEAVY 5 mm LONGITUDINAL WIRES 9 GAUGE CROSS WIRES
L– LADDER TYPE REINFORCEMENT
2– 2 LONGITUDINAL WIRES



GROUND FLOOR PLAN
SCALE 1 : 50

CONCRETE COMPRESSIVE STRENGTH
INTERIOR SLAB ON GRADE – 25 MPa TYPE N
EXTERIOR SLAB ON GRADE – 32 MPa CLASS C2
PERIMETER FOUNDATION WALLS – 25 MPa CLASS F2
GRADE BEAMS – 25 MPa TYPE N

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2			
1	ISSUED FOR INFORMATION	2017/06/23	RW

DESIGNED BY / CONCU PAR	CHECKED BY / VÉRIFIÉ PAR
JC	JC
DRAWN BY / DESSINÉ PAR	SCALE / ÉCHELLE
RW	1:50

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GROUND FLOOR PLAN

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PROJECT NO. / PROJET NO.
1713

SHEET NO. / FEUILLE No.
S101

BEAM BEARING PLATE SCHEDULE

MARK	SIZE	ANCHORS
BBP1	150 x 300 x 16 THK PLATE	2–15M x 400 Lg WELDABLE REBAR
BBP2	130 x 400 x 16 THK PLATE	2–15M x 400 Lg WELDABLE REBAR

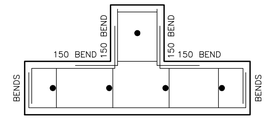
NOTES:
1. BEAMS SUPPORTED ON BEAM BEARING BASE PLATES TO EXTEND ONTO PLATE A MINIMUM OF 80% OF LENGTH OF PLATE, IN DIRECTION OF BEAM UNLESS NOTED OTHERWISE ON PLAN.
2. ANCHORS ARE TO BE CENTERED ON PLATE & SPACED AT 200 o/c TO ALIGN WITH CORE VOIDS IN BLOCK UNLESS OTHERWISE NOTED
3. ENSURE BEAM POCKETS IN MASONRY WALLS ARE BUILT–IN WITH MASONRY OR GROUTED SOLID.
4. ENSURE WALLS ARE GROUTED AT LEAST 3 COURSES BELOW BEAM BEARING PLATES.

BBP1 & BBP2

MASONRY PIER SCHEDULE

MARK	SIZE	REINFORCEMENT
MP1	790 Lg x THICKNESS OF BLOCK	4–20M VERT. + 4–20M DOWEL c/w 1 – 4.88 ø GALVANIZED WIRE TIE @ 200 o/c
MP2	990 x 190	5–20M VERT. + 5–20M DOWEL c/w 1 – 4.88 ø GALVANIZED WIRE TIE @ 200 o/c
MP3	190 x 800+ 190 x 200 (SEE PLAN)	5–20M VERT. + 5–20M DOWEL c/w HEAVY DUTY LADDER TIE @ 200 o/c (SEE DETAIL BELOW)

NOTES:
1. ALL MASONRY PIERS TO BE GROUTED SOLID.
2. ALL MASONRY PIERS TO BE FULL HEIGHT OF WALL



MP3

CONCRETE BLOCK WALL MASONRY LINTEL SCHEDULE UNLESS NOTED WITH ML#

140 OR 190 WD REINF'D CONC BLOCK

SPAN	SIZE & REINF'G
0 – 1000 mm	200 DP 1–15M BOT CONT
1001 – 1900	400 DP 1–20M BOT CONT
1901 – 2500	600 DP 1–20M TOP & BOT CONT
2501 – 3200	800 DP 1–20M TOP & BOT CONT

NOTE:
1. ALL MASONRY LINTELS ARE TO BE GROUTED SOLID
2. GROUT WALL ENDS SOLID BELOW LINTEL FOR WIDTH OF 200 mm LINTEL BEARING
3. SEE DRAWING S00 FOR MASONRY REINFORCEMENT. PROVIDE 1–20M VERT IN EACH GROUTED CORE AT WALL ENDS EACH SIDE OF LINTEL OPENING UNLESS NOTED OTHERWISE ON PLANS.
4. USE LOW WEB BLOCKS FOR LINTEL COURSES CONTAINING HORIZONTAL REBAR

LOAD BEARING MASONRY LINTEL SCHEDULE

MARK	SIZE	REINF'G	BEARING
ML1	190 x 590 DP	1–20M BOTTOM	200

NOTE:
1. ALL LINTELS ARE TO BE GROUTED SOLID
2. GROUT WALL ENDS SOLID BELOW LINTEL FOR WIDTH OF BEARING INDICATED ABOVE.
3. SEE DRAWING S01 & SCHEDULE FOR MASONRY REINFORCEMENT. PROVIDE 1–20M VERT IN EACH GROUTED CORE AT WALL ENDS EACH SIDE OF LINTEL OPENING UNLESS NOTED OTHERWISE ON PLANS.
4. USE SPECIAL LINTEL BLOCKS FOR LOWEST LINTEL COURSE AND USE LOW WEB BLOCKS FOR ALL OTHER LINTEL COURSES.
5. DO NOT INTERRUPT TYPICAL WALL REINFORCEMENT AT LINTELS.

STEEL COLUMN SCHEDULE

MARK	SIZE
SC1	HSS 127 x 127 x 6.4

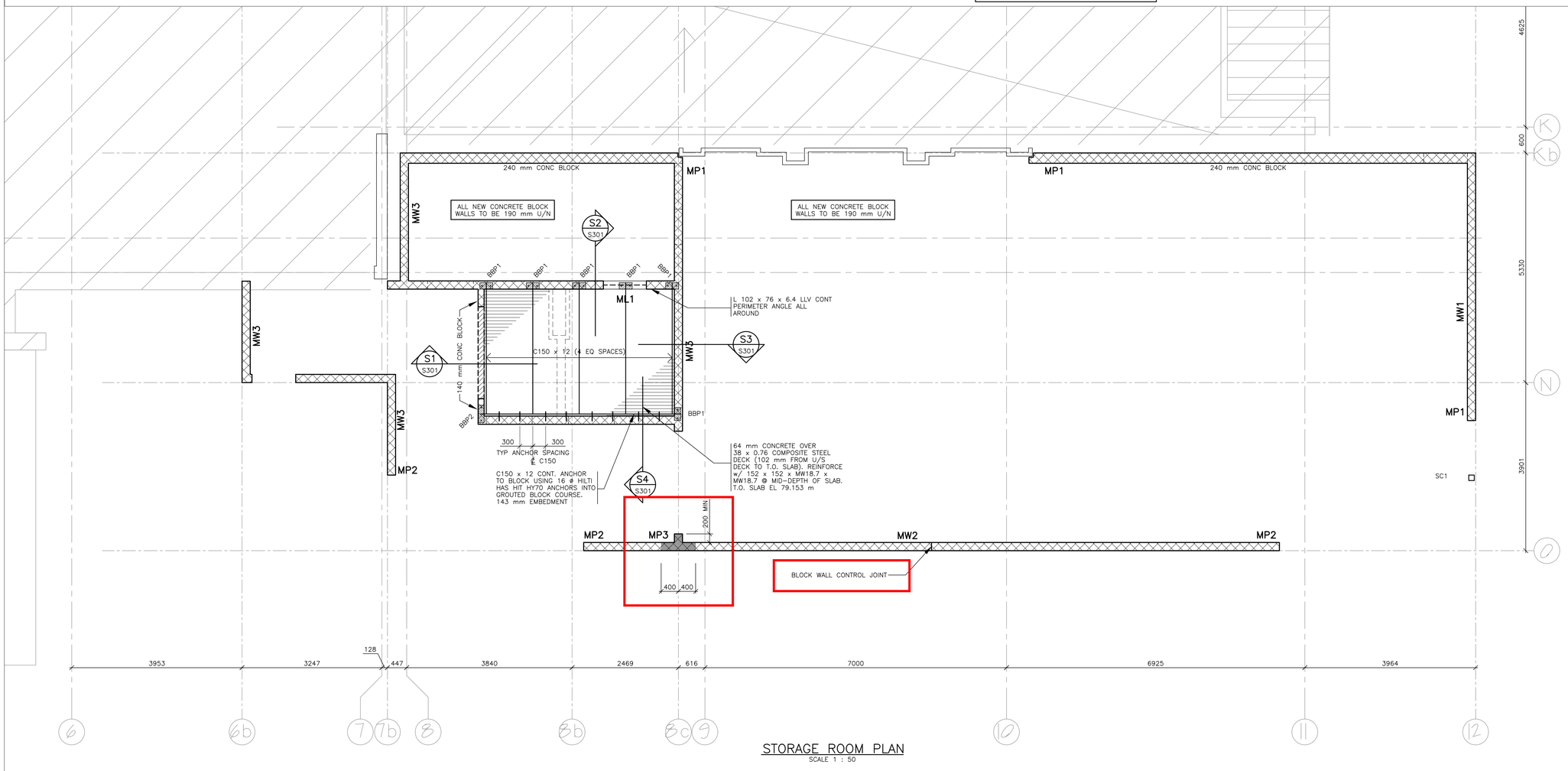
NOTES:
1. ALL HSS SECTIONS TO BE ASTM A500 (GRADE C) OR G40.21M350W (CLASS C).
2. SEE ALSO GENERAL NOTES ON S01.

MASONRY WALL SCHEDULE

MARK	THICKNESS/ GROUTING	REINFORCING VERT.	REINFORCING HORIZ.
MW1	190mm GROUTED @ 600	15M @ 600 o/c	HL2 @ 200 o/c
MW2	190mm GROUTED @ 400	15M @ 400 o/c	HL2 @ 200 o/c
MW3	190mm GROUTED SOLID	15M @ 400 o/c	HL2 @ 200 o/c

NOTE:
1. PROVIDE ADDITIONAL REINFORCING AS FOLLOWS: 1–20M VERT. @ ES OPENINGS, CORNERS & INTERSECTIONS FULLY GROUTED
2. ALL BLOCK WALL SECTIONS EQUAL TO OR LESS THAN 800 mm IN LENGTH TO BE FULLY GROUTED AND REINFORCED WITH 15M VERTICAL @ 200 o/c & HL2 @ 200 o/c (UNLESS NOTED OTHERWISE).
3. SEE PLANS FOR LOCATIONS & EXTENT OF MW #.
4. SEE GENERAL NOTES ON DWG S01 FOR TYPICAL MASONRY WALL REINFORCING UNLESS NOTED OTHERWISE IN SCHEDULE OR ON PLAN.
5. SEE PLANS FOR LOCATIONS OF CONTROL JOINTS (NOTED CJ ON PLAN)

LEGEND
H– HEAVY 5 mm LONGITUDINAL WIRES, 9 GAUGE CROSS WIRES
L– LADDER TYPE REINFORCEMENT
2–2 LONGITUDINAL WIRES



STORAGE ROOM PLAN
SCALE 1 : 50

DESIGN LOADS:

STORAGE ROOM	
64 CONC TOP(102 MAX.)	1.80 kPa
STEEL DECK	0.10
STRUCTURE	0.20
CEILING & FLOORING	0.20
MECH'L/ELECT. & MISC	0.20
DEAD LOAD	2.50 kPa
LIVE LOAD	4.80 kPa
TOTAL LOAD	7.30 kPa

CONCRETE COMPRESSIVE STRENGTH
SLAB ON DECK – 25 MPa TYPE N

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NUMBER/ NUMÉRO	MILESTONE / FAIT SAILLANT	DATE (Y/M/D) (AMJ)	INITIALS INITIALES
6			
5			
4			
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2			
1	ISSUED FOR INFORMATION	2017/06/23	RW

DESIGNED BY / CONCU PAR	CHECKED BY / VÉRIFIÉ PAR
JC	JC
DRAWN BY / DESSINÉ PAR	SCALE / ÉCHELLE
RW	1:50

DETAIL NUMBER 1 / A1.1 SHEET NUMBER — DRAWING TITLE — SCALE:

ARCHITECT / ARCHITECTE

CONSULTANT / EXPERT-CONSEIL

CONSULTANT / EXPERT-CONSEIL

CONSULTANT / EXPERT-CONSEIL

PROJECT / LOCATION / PROJET / ENDROIT

DOVERCOURT
PHASE 2 ADDITION & RENOVATION

411 DOVERCOURT
OTTAWA, ONTARIO

DRAWING / DESSIN

FLOOR PLANS STORAGE ROOM PLAN

FACILITY NO. / NO. DE INSTALLTIONS

SHEET NO. / FEUILLE No.

S102

PROJECT NO. / PROJET NO.

1713

ROOF TOP MECH'L UNIT

RTU
X x Y x Z HIGH
(INCLUDES 305 mm CURB)
WT = X Kg (INCLUDES CURB)

NOTES:
1. REFER TO DWG S02 FOR ADDITIONAL FRAMING BELOW MECH'L UNITS & AT ROOF OPENINGS
2. REFER TO DWG S02 FOR CONCRETE SLAB & CURB DETAILS BELOW MECH'L UNITS
3. COORDINATE MECH'L UNIT OPENINGS WITH MECH'L ENGINEER
4. NOTIFY CUNLIFFE & ASSOCIATES IF ANY OF THE MECH'L UNIT INFORMATION NOTED ON THIS DRAWING DIFFERS FROM THE ACTUAL UNITS SUPPLIED FOR INSTALLATION.
5. 64 CONC SLAB ON 38 x 1.21 COMPOSITE STEEL DECK. REINFORCE SLAB w/ 1 LAYER 152 x 152 x MW18.7 x MW18.7 @ MIDDEPTH OF SLAB. TOP OF SLAB TO FOLLOW ROOF SLOPE. SEE DETAIL ON DRAWING S02 FOR SLAB & CURBS.

STEEL COLUMN SCHEDULE

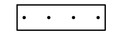
MARK	SIZE
SC1	HSS 127 x 127 x 6.4

NOTES:
1. ALL HSS SECTIONS TO BE ASTM A500 (GRADE C) OR G40.21M350W (CLASS C).
2. SEE ALSO GENERAL NOTES ON S01.

BEAM BEARING PLATE SCHEDULE

MARK	SIZE	ANCHORS
BBP2	130 x 400 x 16 THK PLATE	2–15M x 400 Lg WELDABLE REBAR
BBP3	180 x 350 x 16 THK PLATE	2–15M x 400 Lg WELDABLE REBAR
BBP4	180 x 800 x 25 THK PLATE	4–20M x 800 Lg WELDABLE REBAR
BBP5	180 x 400 x 25 THK PLATE	2–15M x 400 Lg WELDABLE REBAR

NOTES:
1. BEAMS SUPPORTED ON BEAM BEARING BASE PLATES TO EXTEND ONTO PLATE A MINIMUM OF 80% OF LENGTH OF PLATE, IN DIRECTION OF BEAM UNLESS NOTED OTHERWISE ON PLAN.
2. ANCHORS ARE TO BE CENTERED ON PLATE & SPACED AT 200 o/c TO ALIGN WITH CORE VOIDS IN BLOCK UNLESS OTHERWISE NOTED
3. ENSURE BEAM POCKETS IN MASONRY WALLS ARE BUILT–IN WITH MASONRY OR GROUTED SOLID.
4. ENSURE WALLS ARE GROUTED AT LEAST 3 COURSES BELOW BEAM BEARING PLATES.



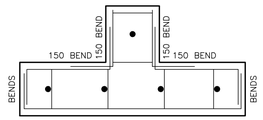
BBP2, BBP3, BBP5

BBP4

MASONRY PIER SCHEDULE

MARK	SIZE	REINFORCEMENT
MP1	790 Lg x THICKNESS OF BLOCK	4–20M VERT. + 4–20M DOWEL c/w 1 – 4.88 ø GALVANIZED WIRE TIE @ 200 o/c
MP2	990 x 190	5–20M VERT. + 5–20M DOWEL c/w 1 – 4.88 ø GALVANIZED WIRE TIE @ 200 o/c
MP3	190 x 800+ 190 x 200 (SEE PLAN)	5–20M VERT. + 5–20M DOWEL c/w HEAVY DUTY LADDER TIE @ 200 o/c (SEE DETAIL BELOW)

NOTES:
1. ALL MASONRY PIERS TO BE GROUTED SOLID.
2. ALL MASONRY PIERS TO BE FULL HEIGHT OF WALL



MP3

CONCRETE BLOCK WALL MASONRY LINTEL SCHEDULE UNLESS NOTED WITH ML#

140 OR 190 WD REINF'D CONC BLOCK

SPAN	SIZE & REINF'G
0 – 1000 mm	200 DP 1–15M BOT CONT
1001 – 1900	400 DP 1–20M BOT CONT
1901 – 2500	600 DP 1–20M TOP & BOT CONT
2501 – 3200	800 DP 1–20M TOP & BOT CONT

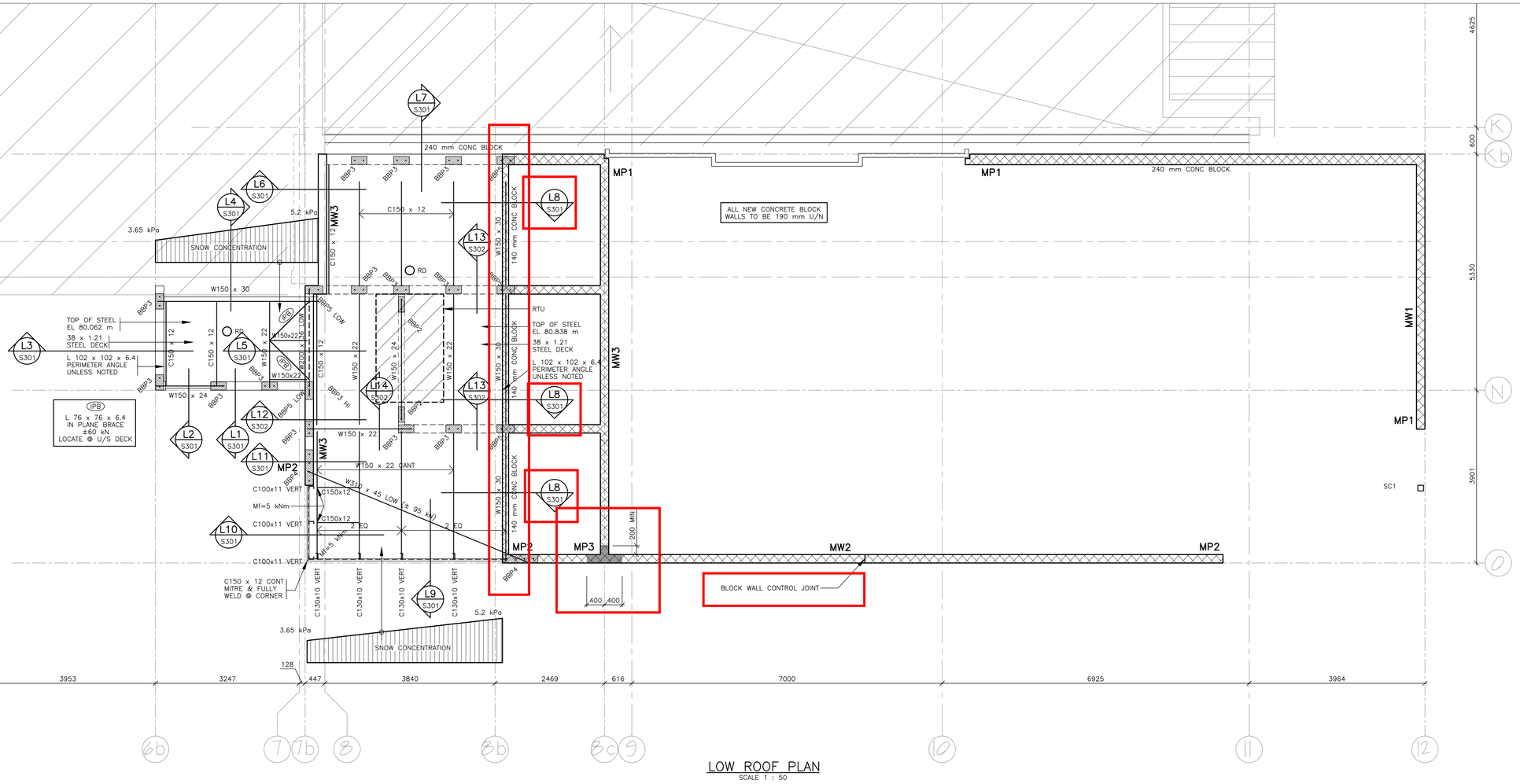
NOTE:
1. ALL MASONRY LINTELS ARE TO BE GROUTED SOLID
2. GROUT WALL ENDS SOLID BELOW LINTEL FOR WIDTH OF 200 mm LINTEL BEARING
3. SEE DRAWING S00 FOR MASONRY REINFORCEMENT. PROVIDE 1–20M VERT IN EACH GROUTED CORE AT WALL ENDS EACH SIDE OF LINTEL OPENING UNLESS NOTED OTHERWISE ON PLANS.
4. USE LOW WEB BLOCKS FOR LINTEL COURSES CONTAINING HORIZONTAL REBAR

MASONRY WALL SCHEDULE

MARK	THICKNESS/ GROUTING	REINFORCING VERT.	REINFORCING HORIZ.
MW1	190mm GROUTED @ 600	15M @ 600 o/c	HL2 @ 200 o/c
MW2	190mm GROUTED @ 400	15M @ 400 o/c	HL2 @ 200 o/c
MW3	190mm GROUTED SOLID	15M @ 400 o/c	HL2 @ 200 o/c

NOTE:
1. PROVIDE ADDITIONAL REINFORCING AS FOLLOWS: 1–20M VERT. @ ES OPENINGS, CORNERS & INTERSECTIONS FULLY GROUTED
2. ALL BLOCK WALL SECTIONS EQUAL TO OR LESS THAN 800 mm IN LENGTH TO BE FULLY GROUTED AND REINFORCED WITH 15M VERTICAL @ 200 o/c & HL2 @ 200 o/c (UNLESS NOTED OTHERWISE).
3. SEE PLANS FOR LOCATIONS & EXTENT OF MW #.
4. SEE GENERAL NOTES ON DWG S01 FOR TYPICAL MASONRY WALL REINFORCING UNLESS NOTED OTHERWISE IN SCHEDULE OR ON PLAN.
5. SEE PLANS FOR LOCATIONS OF CONTROL JOINTS (NOTED CJ ON PLAN)

LEGEND
H– HEAVY 5 mm LONGITUDINAL WIRES
9 GAUGE CROSS WIRES
L– LADDER TYPE REINFORCEMENT
2– 2 LONGITUDINAL WIRES



LOW ROOF PLAN
SCALE 1 : 50

NOTE: (AXIAL COLLECTOR LOADS NOTED ON PLANS AS ±#kN)

DESIGN AXIAL COLLECTOR LOADS AND ROLL OVER FORCES (ROF) NOTED ON PLAN HAVE BEEN MULTIPLIED BY Rd=1.5

CONCRETE COMPRESSIVE STRENGTH
SLAB ON DECK – 25 MPa TYPE N

DESIGN LOADS:

TYPICAL STEEL ROOF AREAS	
ROOF'G & INSUL.	0.65 kPa
BOARD	0.10
STEEL DECK	0.15
STRUCTURE	0.20
MECH. & MISC.	0.40
DEAD LOAD	1.50 kPa
SNOW LOAD	2.32 kPa (OR CONCENTRATION)
TOTAL LOAD	3.82 kPa (OR DL + CONCEN)

MECHANICAL EQUIPMENT AREAS	
CONCRETE CURBS	4.50 kPa
CONCRETE PAD	2.40
ROOF'G & INSUL.	0.60
STEEL DECK	0.15
STRUCTURE	0.25
CEILING	0.15
MECH. & MISC.	2.80
DEAD LOAD	10.85 kPa
SNOW LOAD	2.32 kPa (OR CONCENTRATION)
TOTAL LOAD	13.17 kPa U/N (OR DL + CONCEN)

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6			
5			
4			
3			
2			
1	ISSUED FOR INFORMATION	2017/06/23	RW

DESIGNED BY / CONCU PAR	CHECKED BY / VERIFIÉ PAR
JC	JC
DRAWN BY / DESSINÉ PAR	SCALE / ÉCHELLE
RW	1:50

DETAIL NUMBER 1 / A1.1 SHEET NUMBER — DRAWING TITLE — SCALE:

ARCHITECT / ARCHITECTE	*CONSULTANT / EXPERT-CONSEIL*
CONSULTANT / EXPERT-CONSEIL	*CONSULTANT / EXPERT-CONSEIL*

PROJECT / LOCATION / PROJET / ENDROIT

DOVERCOURT
PHASE 2 ADDITION & RENOVATION

411 DOVERCOURT
OTTAWA, ONTARIO

DRAWING / DESSIN

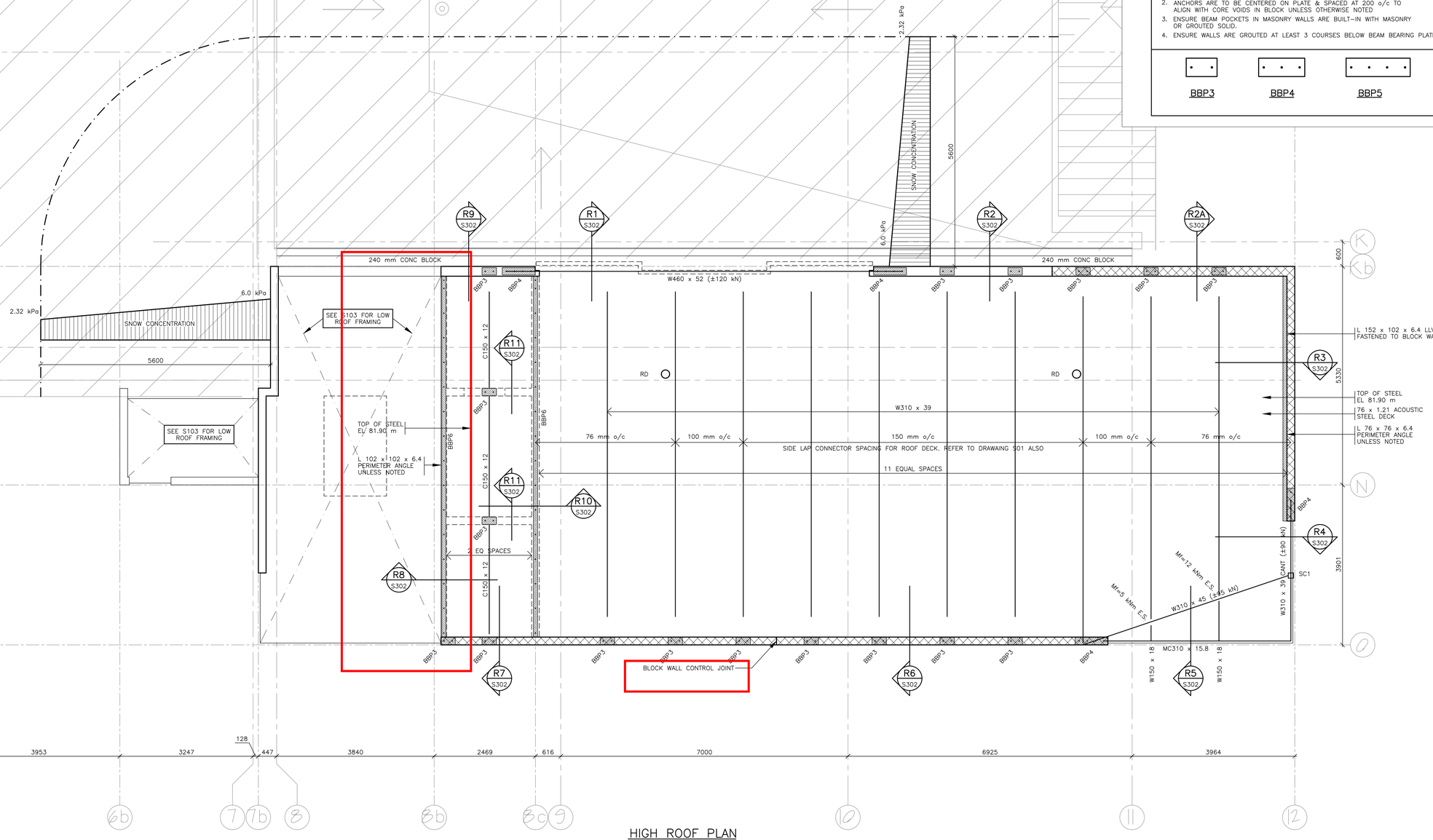
FLOOR PLANS LOW ROOF PLAN

FACILITY NO. / NO. DE INSTALLTIONS	*SHEET NO. / FEUILLE No.*
PROJECT NO. / PROJET NO. 1713	S103

BEAM BEARING PLATE SCHEDULE		
MARK	SIZE	ANCHORS
BBP3	180 x 350 x 16 THK PLATE	2–15M x 400 Lg WELDABLE REBAR
BBP4	180 x 800 x 25 THK PLATE	4–20M x 800 Lg WELDABLE REBAR
BBP5	180 x 400 x 25 THK PLATE	2–15M x 400 Lg WELDABLE REBAR
BBP6	100 x 12 THK PLATE CONT	1–15M x 400 Lg WELDABLE REBAR SPACED @ 600 o/c

NOTES:
1. BEAMS SUPPORTED ON BEAM BEARING BASE PLATES TO EXTEND ONTO PLATE A MINIMUM OF 80% OF LENGTH OF PLATE, IN DIRECTION OF BEAM UNLESS NOTED OTHERWISE ON PLAN.
2. ANCHORS ARE TO BE CENTERED ON PLATE & SPACED AT 200 o/c TO ALIGN WITH CORE VOIDS IN BLOCK UNLESS OTHERWISE NOTED
3. ENSURE BEAM POCKETS IN MASONRY WALLS ARE BUILT–IN WITH MASONRY OR GROUTED SOLID.
4. ENSURE WALLS ARE GROUTED AT LEAST 3 COURSES BELOW BEAM BEARING PLATES.

BBP3 BBP4 BBP5



HIGH ROOF PLAN
SCALE 1 : 50

NOTE: (AXIAL COLLECTOR LOADS NOTED ON PLANS AS ±#kN)

DESIGN AXIAL COLLECTOR LOADS NOTED ON PLAN HAVE BEEN MULTIPLIED BY Rd=1.5

DESIGN LOADS:

TYPICAL STEEL ROOF AREAS	
ROOF'G & INSUL.	0.65 kPa
BOARD	0.10
STEEL DECK	0.15
STRUCTURE	0.20
MECH. & MISC.	0.40
DEAD LOAD	1.50 kPa
SNOW LOAD	2.32 kPa (OR CONCENTRATION)
TOTAL LOAD	3.82 kPa (OR DL + CONCEN)

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NUMBER / NUMÉRO	MILESTONE / FAIT SAILLANT	DATE (Y/M/D) (AA/M/J)	INITIALS INITIALES
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1	ISSUED FOR INFORMATION	2017/06/23	RW

DESIGNED BY / CONCU PAR: JC
CHECKED BY / VERIFIÉ PAR: JC
DRAWN BY / DESSINÉ PAR: RW
SCALE / ÉCHELLE: 1:50

DETAIL NUMBER 1 / SHEET NUMBER A1.1 — DRAWING TITLE / SCALE:

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CONSULTANT / EXPERT-CONSEIL

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CONSULTANT / EXPERT-CONSEIL

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DOVERCOURT
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411 DOVERCOURT
OTTAWA, ONTARIO

DRAWING / DESSIN

FLOOR PLANS HIGH ROOF PLAN

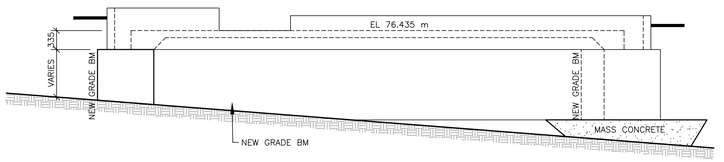
FACILITY NO. / NO. DE INSTALLTIONS

SHEET NO. / FEUILLE No.

S104

PROJECT NO. / PROJET NO.

1713



F S100 SECTION
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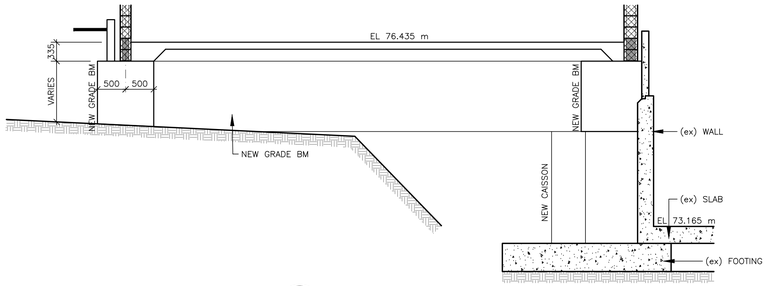

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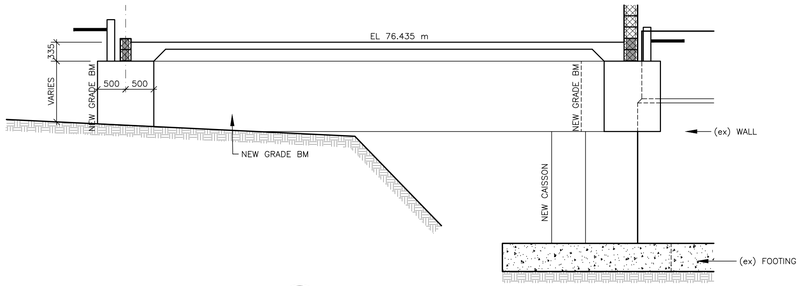

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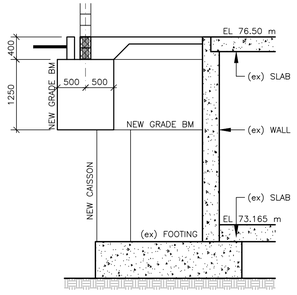

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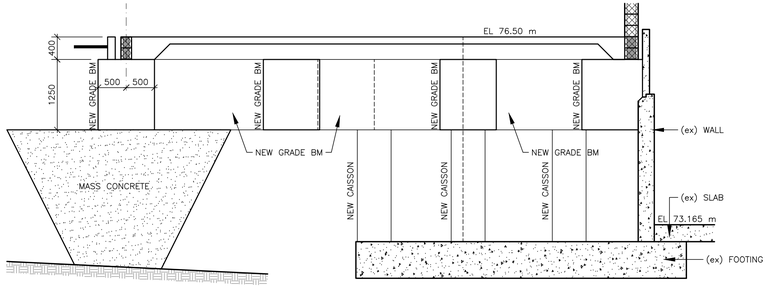

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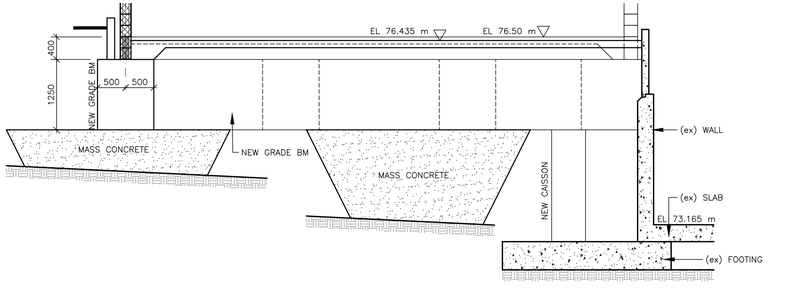

C S100 SECTION
1 : 50

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DRAWN BY / DESSINÉ PAR: RW	SCALE / ÉCHELLE: 1:50

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OTTAWA, ONTARIO

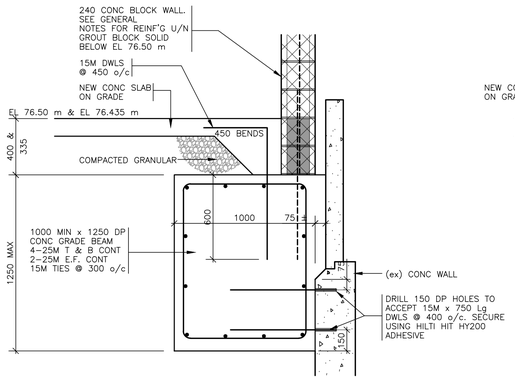
DRAWING / DESSIN

SECTIONS

FACILITY NO. / NO. DE INSTALLTIONS

PROJECT NO. / PROJET NO.
1713

SHEET NO. / FEUILLE No.
S200



9 S100 SECTION
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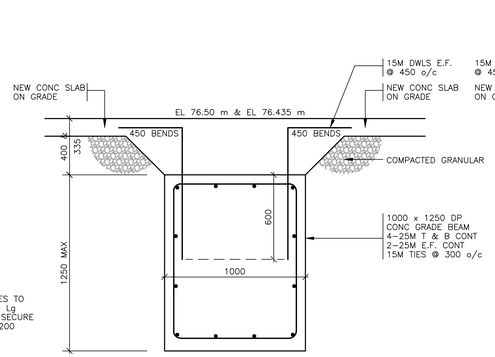
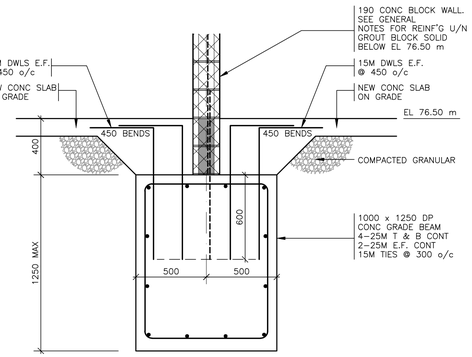
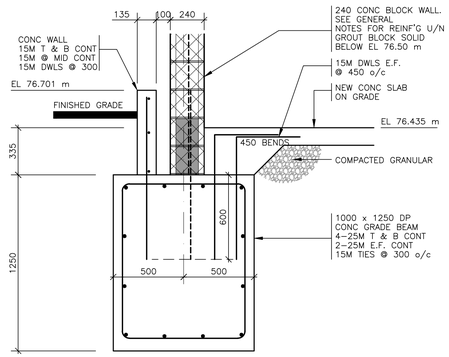


TYPICAL GRADE BEAM SECTION
w/ NO BLOCK WALL ABOVE
SCALE 1 : 20



TYPICAL GRADE BEAM SECTION
@ INTERIOR BLOCK WALL
SCALE 1 : 20



8 S100 SECTION
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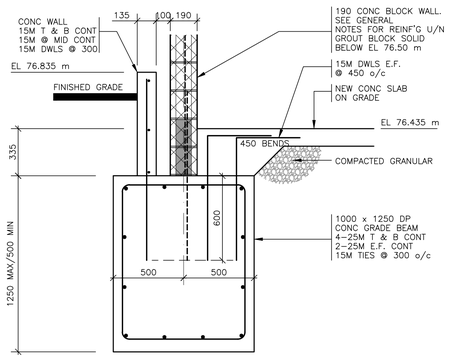

4 S100 SECTION
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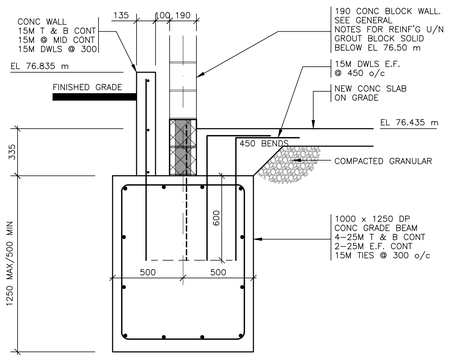

5 S100 SECTION
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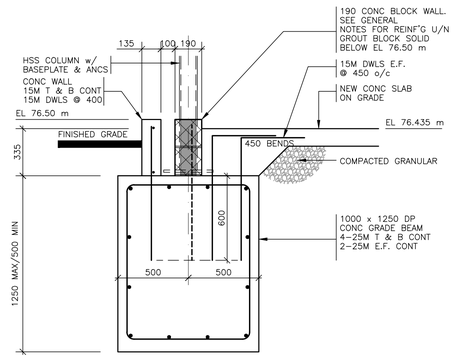
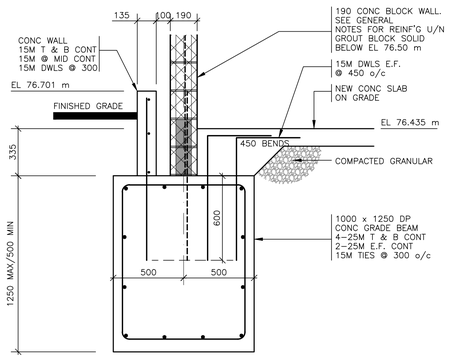


6 S100 SECTION
1 : 20



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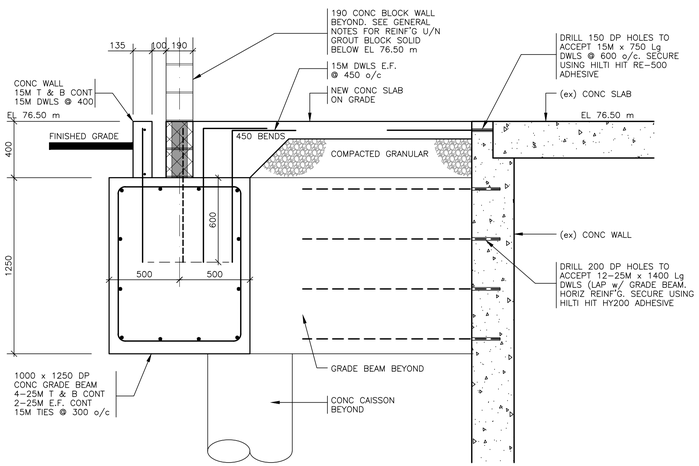
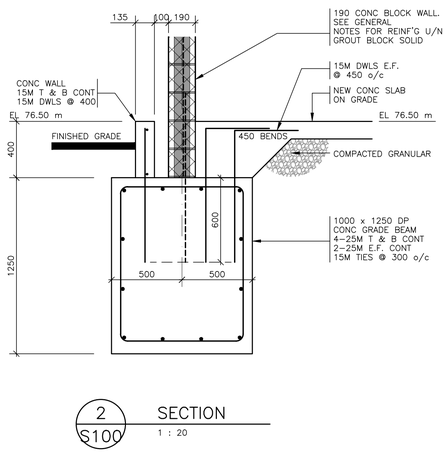


1 S100 SECTION
1 : 20



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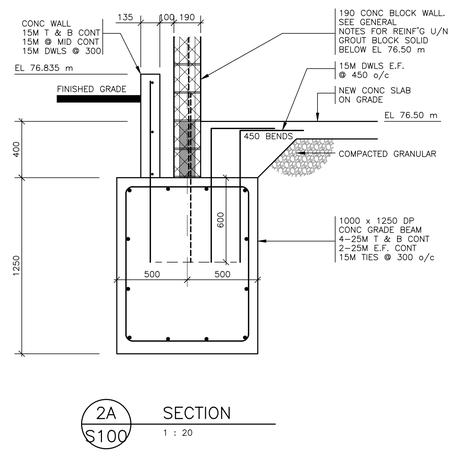
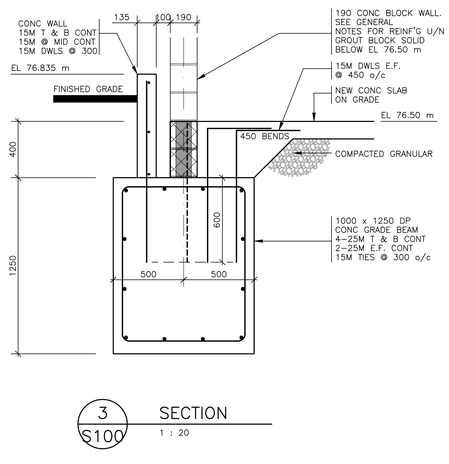
1 : 20



2A S100 SECTION
1 : 20



3 S100 SECTION
1 : 20

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NUMBER/ NUMÉRO	MILESTONE / FAIT SAILLANT	DATE (Y/M/D) (AA/M/J)	INITIALS INITIALES
6			
5			
4			
3			
2			
1	ISSUED FOR INFORMATION	2017/06/23	RW

DESIGNED BY / CONCU PAR	CHECKED BY / VERIFIÉ PAR
JC	JC
DRAWN BY / DESSINÉ PAR	SCALE / ÉCHELLE
RW	1:50

DETAIL NUMBER / DRAWING TITLE / SCALE / SHEET NUMBER

ARCHITECT / ARCHITECTE	*CONSULTANT / EXPERT-CONSEIL*
CONSULTANT / EXPERT-CONSEIL	*CONSULTANT / EXPERT-CONSEIL*

PROJECT / LOCATION / PROJET / ENDROIT

DOVERCOURT

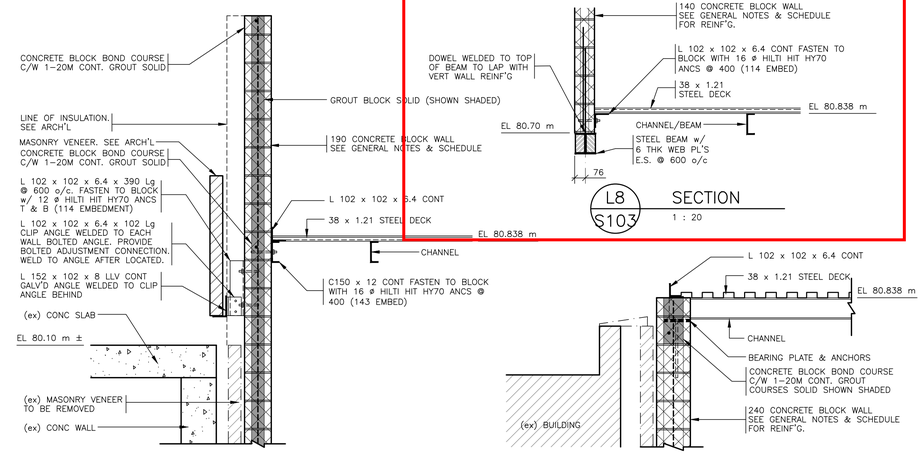
PHASE 2
ADDITION & RENOVATION

411 DOVERCOURT
OTTAWA, ONTARIO

DRAWING / DESSIN

SECTIONS & DETAILS

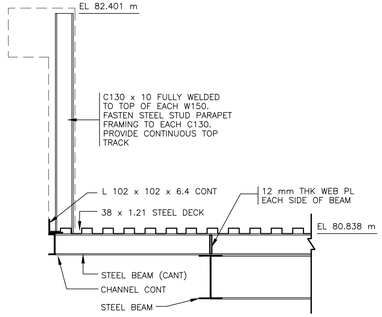
FACILITY NO. / NO. DE INSTALLTIONS	*SHEET NO. / FEUILLE No.*
PROJECT NO. / PROJET NO. 1713	S300



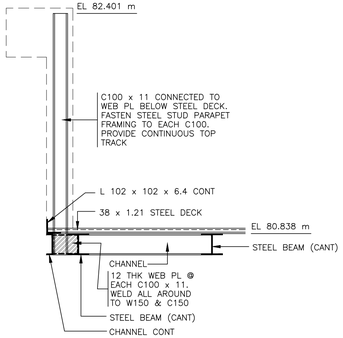
L6 S103 SECTION 1 : 20

L8 S103 SECTION 1 : 20

L7 S103 SECTION 1 : 20



L9 S103 SECTION 1 : 20



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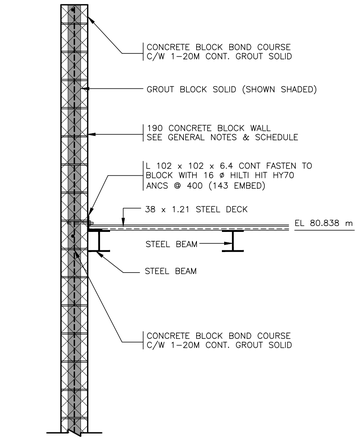

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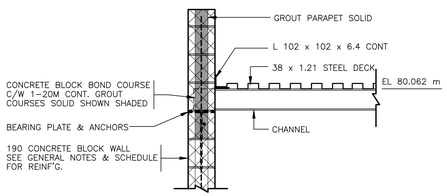
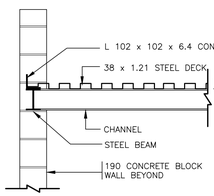
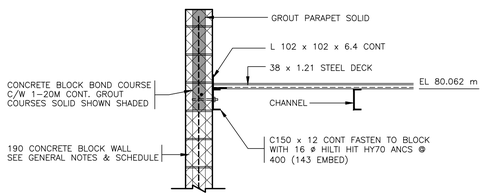


L1 S103 SECTION 1 : 20



L2 S103 SECTION 1 : 20



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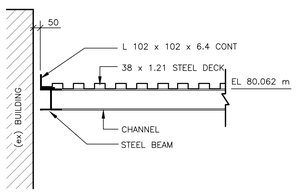

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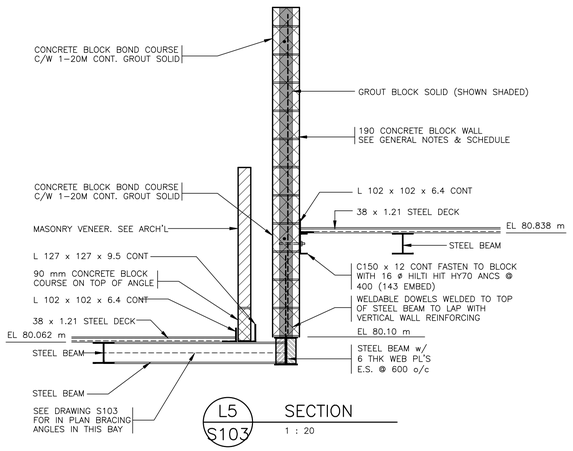

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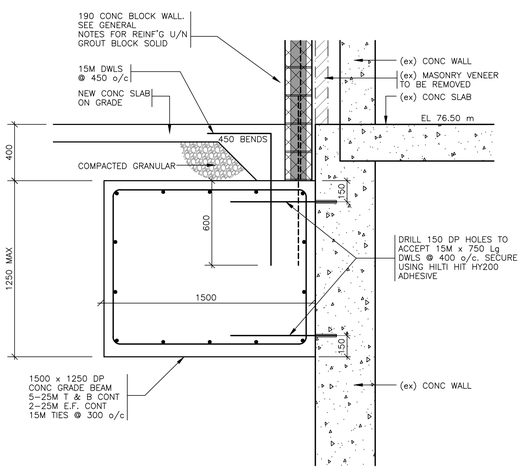
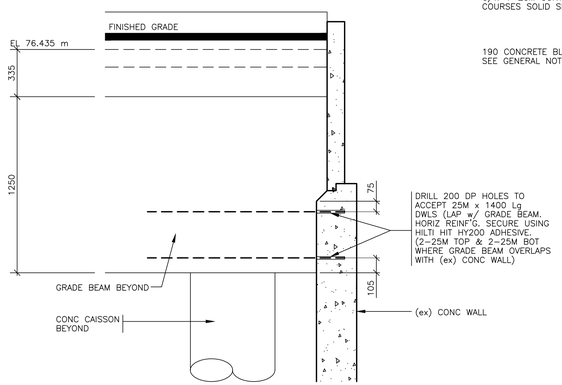
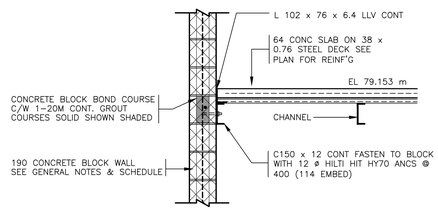


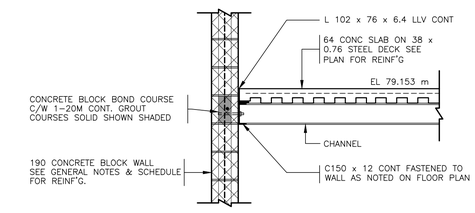
10 S100 SECTION 1 : 20



11 S100 SECTION 1 : 20



S3 S102 SECTION 1 : 20



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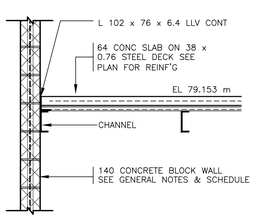
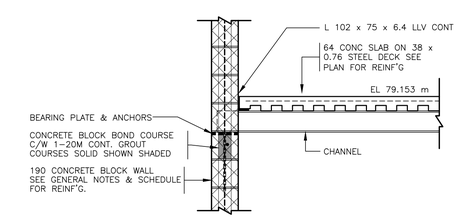


S1 S102 SECTION 1 : 20



S2 S102 SECTION 1 : 20

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1	ISSUED FOR INFORMATION	2017/06/23	RW

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JC	JC
DRAWN BY / DESSINÉ PAR	SCALE / ÉCHELLE
RW	1:50

DETAIL NUMBER 1 / A1.1 SHEET NUMBER — DRAWING TITLE / SCALE:

ARCHITECT / ARCHITECTE	*CONSULTANT / EXPERT-CONSEIL*
CONSULTANT / EXPERT-CONSEIL	*CONSULTANT / EXPERT-CONSEIL*

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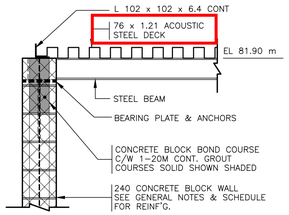
SECTIONS & DETAILS

FACILITY NO. / NO. DE INSTALLTIONS

PROJECT NO. / PROJET NO.
1713

SHEET NO. / FEUILLE No.
S301



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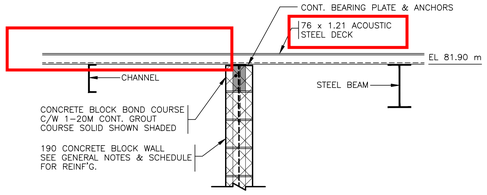

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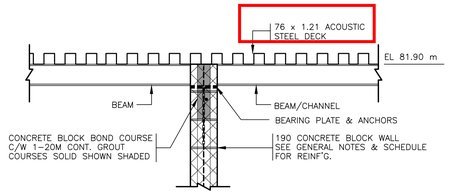

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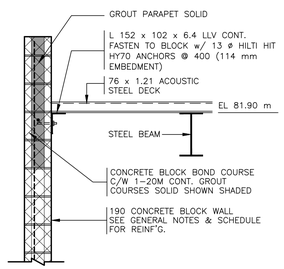

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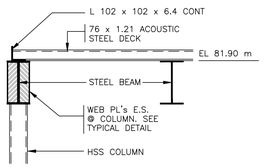

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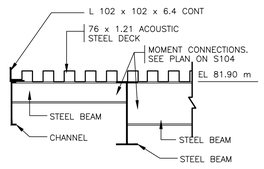

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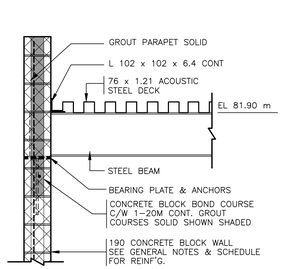

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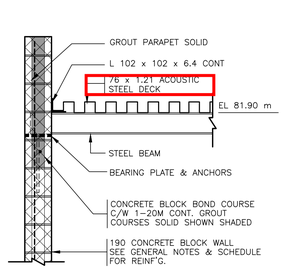

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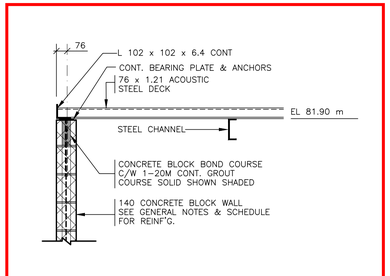
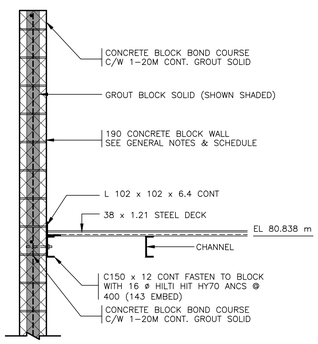
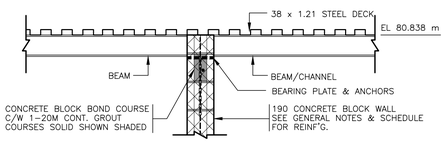


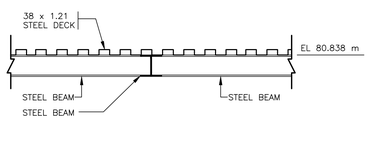
R8 S104 SECTION 1 : 20



L12 S103 SECTION 1 : 20



L13 S103 SECTION 1 : 20



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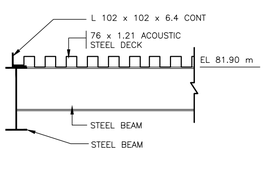

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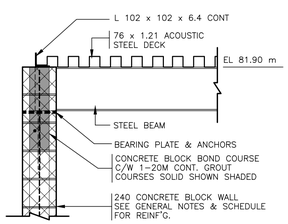

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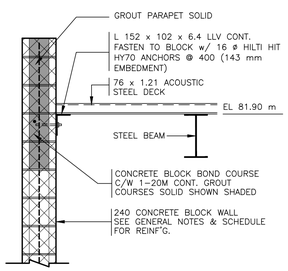

R2A S104 SECTION 1 : 20

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DESIGNED BY / CONCU PAR: JC	CHECKED BY / VÉRIFIÉ PAR: JC
DRAWN BY / DESSINÉ PAR: RW	SCALE / ÉCHELLE: 1:50

DETAIL NUMBER 1 / A1.1 SHEET NUMBER — DRAWING TITLE — SCALE:

ARCHITECT / ARCHITECTE

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SECTIONS & DETAILS

FACILITY NO. / NO. DE INSTALLTIONS

PROJECT NO. / PROJET NO.
1713

SHEET NO. / FEUILLE No.
S302