SEISMIC SYSTEM/LOADING DATA:

MAIN BUILDING

SEISMIC FORCE RESISTING SYSTEM (SFRS)

SFRS: SYSTEM & CONNECTIONS: (2012 OBC CLAUSE 4.1.8.9/4.1.8.10)
LATERAL LOAD RESISTING SYSTEM: CONVENTIONAL CONSTRUCTION BRACED FRAMES,WOOD SHEARWALLS
Rd = 1.5
Ro = 1.3

CSA STANDARD:	CAN/CSA S16-09	CAN/CSA 086-09
APPLICABLE CLAUSE(S):	27.11	9

SFRS: DIAPHRAGMS & CONNECTIONS: (2012 OBC CLAUSE 4.1.8.15)

CSA STANDARD:	CAN/CSA 086-09
APPLICABLE CLAUSE(S):	9

SFRS: SYSTEM FOUNDATIONS: (2012 OBC CLAUSE 4.1.8.16)
CSA STANDARD: CAN/CSA A23.3-04 ☐ FOR ANCHORED FOOTINGS
APPLICABLE CLAUSE(S): 21.11.1.2 (e) X FOR UNANCHORED FOOTINGS
CONFIRMATION: FOUNDATIONS HAVE BEEN DESIGNED TO RESIST THE LATERAL LOAD CAPACITY OF THE SFRS INCLUDING ALL APPLICABLE AMPLIFICATION FACTORS

SEISMIC IMPORTANCE FACTOR: (2012 OBC CLAUSE 4.1.8.5)
Ie = 1.0

PROJECT CITY: ARNPRIOR ONTARIO

SITE CLASS: THE NOTED SITE CLASSIFICATION FOR SEISMIC SITE RESPONSE AND SHEAR WAVE VELOCITY PARAMETERS INDICATED ARE AS REPORTED IN THE GEOTECHNICAL REPORT # 64356.01 BY HOULE CHEVRIER ENGINEERING REFER TO THE NOTED GEOTECHNICAL REPORT FOR V's, N60, AND/OR Su VALUES USED TO DETERMINE SITE CLASSIFICATION.

☐ A ☐ B ☐ C X D ☐ E ☐ F (SITE SPECIFIC SPECTRUM: ARNPRIOR ONTARIO)

PGA: 0.310

RESPONSE SPECTRUM DATA:

5% DAMPED SPECTRAL RESPONSE ACCELERATION VALUES: (2012 OBC SUPPLEMENT STANDARD SB-1)

Sa (0.2) = 0.610
Sa (0.5) = 0.290
Sa (1.0) = 0.130
Sa (2.0) = 0.044

DESIGN SPECTRAL RESPONSE ACCELERATION VALUES (DSRAV): (2012 OBC CLAUSE 4.1.8.4)

X CLASS D: (Fa=1.156/Fv=1.37)

S (0) = 0.705
S (0.2) = 0.705
S (0.5) = 0.397
S (1.0) = 0.178
S (2.0) = 0.060
S (4.0) = 0.030

SYSTEM RESTRICTION VALUE: IeFaSa(0.2) = 0.705 ≥ 0.35 X YES ☐ NO

PERIOD DATA:

STATIC PERIOD: (2012 OBC CLAUSE 4.1.8.11(3))

Ta (STATIC) NS = 0.125 sec
Ta (STATIC) EW = 0.125 sec

DESIGN PERIODS/MODE & MOMENT FACTORS: (2012 OBC CLAUSE (4.1.8.11(5))

Sa(0.2) / Sa(2.0) = 13.9 ≥ 8.0 X YES ☐ NO

Ta (DESIGN) NS = 0.125 sec	MV = 1.00	J = 1.00
Ta (DESIGN) EW = 0.125 sec	MV = 1.00	J = 1.00

DESIGN FUNDAMENTAL PERIOD BASED DSRAV:

S(Ta) NS = 0.705
S(Ta) EW = 0.705

IRREGULARITY REVIEW (2012 OBC CLAUSE 4.1.8.6)

1.	VERTICAL STIFFNESS:	☐ YES	X NO
2.	WEIGHT:	☐ YES	X NO
3.	VERTICAL GEOMETRIC:	☐ YES	X NO
4.	IN PLANE DISCONTINUITY:	☐ YES	X NO
5.	OUT OF PLANE:	☐ YES	X NO
6.	WEAK STOREY:	☐ YES	X NO
7.	TORSIONAL:	☐ YES	X NO
	B NS = N/A (FLEXIBLE DIAPHRAGM)		
	B EW = N/A		
8.	NON-ORTHAGONAL:	☐ YES	X NO

CONCLUSION: BUILDING IS X REGULAR ☐ IRREGULAR
DYNAMIC ANALYSIS: ☐ REQUIRED X NOT REQUIRED
DYNAMIC PROCEDURE METHOD: ☐ MODAL RESPONSE SPECTRUM ☐ NUMERICAL INTEGRATION TIME HISTORY X N/A

TORSIONAL ECCENTRICITY:
X ± 0.10 Dnx (4.1.8.11(10a), B ≤ 1.7 EQUIV. STATIC FORCE PROCEDURE)
☐ ± 0.10 Dnx (4.1.8.12(4a), B ≥ 1.7)
☐ ± 0.05 Dnx (4.1.8.12(4b), B < 1.7, 3-D DYNAMIC ANALYSIS)

STRUCTURAL SEPARATION:
☐ THE NEW AND EXISTING STRUCTURES HAVE BEEN SEPARATED IN ACCORDANCE WITH 4.1.8.14(1) OF THE 2012 O.B.C.
X N/A

BASE SHEARS/MOMENTS (2012 OBC CLAUSE 4.1.8.11)

Vstatic = S(Ta)MvIeW/(RdRo) = 974 kN W = 2695 kN

STATIC MAXIMUM/MINIMUM VALUES:

NORTH-SOUTH: (↑)

Vmin = S(2.0)MvIeW/(RdRo) = 83 kN W = 2695 kN
Vmax = 2/3 S(0.2)IeW/(RdRo) = 650 kN W = 2695 kN

EAST-WEST: (↔)

Vmin = S(2.0)MvIeW/(RdRo) = 83 kN W = 2695 kN
Vmax = 2/3 S(0.2)IeW/(RdRo) = 650 kN W = 2695 kN

G E N E R A L N O T E S

1. ANY DEVIATION FROM THE CONDITIONS SHOWN ON THESE DRAWINGS MUST BE REPORTED TO THE ENGINEER.
2. THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF PART 4 OF THE O.B.C. (2012 EDITION) ONTARIO REGULATION 332/12 (AS AMENDED)
3. STANDARDS
   -CSA STANDARD A23.3-04 DESIGN OF CONCRETE STRUCTURES
   -CAN/CSA-S16-09 LIMIT STATES DESIGNS OF STEEL STRUCTURES
   -CSA STANDARD S304.1-04 DESIGN OF MASONRY STRUCTURES
   -CAN/CSA-O86-09 ENGINEERING DESIGN IN WOOD
4. ANY MODIFICATIONS TO EXISTING STRUCTURES ARE TO BE LIMITED TO WORK NOTED ON THESE DRAWINGS. ANY ADDITIONAL OR PROPOSED MODIFICATIONS TO EXISTING STRUCTURES MUST BE APPROVED BY THE ENGINEER
5. FOUNDATIONS
   .1 ALL FOOTINGS ARE TO BEAR ON NATURAL UNDISTURBED SOIL OR ENGINEERED FILL.
   .2 BEARING CAPACITY USED IN THE FOOTING DESIGN IS ASSUMED TO BE SLS= 150 kPa/ULS=300 kPa
   .3 BEARING SURFACE IS TO BE INSPECTED BY GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE.
   .4 FOR FURTHER INFORMATION SEE GEOTECHNICAL REPORT No. 64356.01 PREPARED BY HOULE CHEVRIER ENGINEERING
   .5 STEP FOOTINGS WHERE INDICATED ON PLAN AT THE RATE OF 2 HORIZONTAL TO 1 VERTICAL.
6. SLABS ON GRADE
   .1 SLABS ON GRADE TO BE UNREINFORCED UNLESS NOTED.
   .2 FOR COMPOSITION & COMPACTION OF FILL SUPPORTING SLABS ON GRADE SEE GEOTECHNICAL REPORT.
   .3 PROVIDE 12 mm ASPHALT IMPREGNATED FIBREBOARD BETWEEN SLABS ON GRADE & FOUNDATION WALLS OR COLUMNS.
   .4 SAWCUT SLAB ON GRADE TO (1/4 x SLAB DEPTH) 8 HOURS AFTER CONCRETE PLACEMENT.
   .5 SPACE SAWCUTS ON A 4500 mm x 4500 mm MAXIMUM GRID. AVOID LONG & NARROW SAWCUT PATTERNS. LOCATE SAWCUTS ALONG COLUMN LINES WHERE POSSIBLE. CONTRACTOR IS TO PROVIDE THE ENGINEER WITH DOCUMENTATION SHOWING PROPOSED SAWCUT LOCATIONS FOR APPROVAL UNLESS SAWCUTS LOCATIONS ARE OTHERWISE INDICATED ON THESE DRAWINGS.
7. MATERIALS
   .1 CONCRETE STRENGTH AT 28 DAYS TO BE AS NOTED ON THESE DRAWINGS AND SPECIFICATIONS.
   .2 REINFORCING STEEL TO BE DEFORMED GRADE 400R WITH Fy= 400 MPa.
   .3 HOLLOW STRUCTURAL STEEL SECTIONS TO BE ASTM A500 GRADE C OR G40.21 350W CLASS C.
   .4 ALL 'W' & 'WWF' SHAPE STEEL SECTIONS TO BE GRADE G40.21 350W WITH Fy= 350 MPa.
   .5 ALL OTHER STRUCTURAL STEEL TO BE GRADE G40.21 300W WITH Fy= 300 MPa UNLESS NOTED OTHERWISE.
   .6 ALL STRUCTURAL STEEL TO RECEIVE 1 SHOP APPLIED COAT OF PRIMER UNLESS NOTED.
   .7 ALL STRUCTURAL STEEL EXPOSED TO EXTERIOR IS TO BE HOT DIP GALVANIZED UNLESS NOTED.
   .8 ANCHOR BOLTS TO BE A307.
   .9 ALL OTHER BOLTS TO BE A325.
   .10 A325 BOLTS EXPOSED TO EXTERIOR ARE TO BE STAINLESS STEEL
   .11 A307 BOLTS EXPOSED TO EXTERIOR ARE TO BE GALVANIZED.
   .12 CONCRETE BLOCK TO BE H/15/A/M
   .13 CONCRETE BLOCK MASONRY MORTAR TO BE 8.5 MPa TYPE 'S' U/N.
   .14 CONCRETE BLOCK MASONRY GROUT TO BE 12 MPa "HIGH SLUMP" (200-250 mm SLUMP)
   .15 ALL WOOD STUDS TO BE SPF No. 2 OR BETTER
   .16 ALL PLYWOOD TO BE D.FIR PLYWOOD TO CSA 0121 OR CANADIAN SOFTWOOD PLYWOOD TO 0151
   .17 ALL OSB TO MEET CSA 0325.
8. CONCRETE COVER
   .1 FOOTINGS 75 mm BOTTOM, 50 mm SIDES
   .2 WALLS 40 mm UNLESS NOTED OTHERWISE
   .3 PIERS 40 mm
9. REINFORCING STEEL DESIGNATION
   8-20M x 1500 T/B
   8 = NUMBER OF BARS
   20M = SIZE OF BARS
   1500 = LENGTH OF BARS
   T = BAR LOCATION - TOP
   B = BAR LOCATION - BOT
   LENGTH OF BARS DOES NOT INCLUDE HOOKS OR BENDS
10. DOWELS
    DOWELS TO FOOTINGS TO BE OF SAME DIAMETER AS THE LOWEST LIFT OF VERTICAL REINFORCING IN COLUMNS, PIERS OR WALLS.
11. REINFORCING STEEL SPLICES
    REINFORCING STEEL SPLICES TO BE AS NOTED IN REINFORCING BAR LAP LENGTH TABLE ON S01 U/N.
12. OPENINGS
    .1 AT OPENINGS IN WALLS PROVIDE 2-20M T & B OF OPENING EXTENDING 600 mm MIN. BEYOND CORNERS OF OPENINGS.
    .2 FOR ADDITIONAL OPENINGS 300 x 300 OR SMALLER SEE ARCHITECTURAL & MECHANICAL DRAWINGS.
    .3 REPORT ANY OPENINGS LARGER THAN 300 x 300 NOT SHOWN ON THESE DRAWINGS TO THE ENGINEER.
13. LOADS
    ALL LOADS & FORCES INDICATED ON THESE DRAWINGS ARE UNFACTORED WORKING LOADS UNLESS NOTED.
14. CONCRETE BLOCK MASONRY
    .1 140 mm CONCRETE BLOCK:
    VERT: 1-15M @ 800 o/c
    HORIZ: SL2 @ 200 o/c OR HL2 @ 400 o/c
    .2 190 mm CONCRETE BLOCK
    VERT: 1-15M @ 800 o/c
    HORIZ: HL2 @ 200 o/c

    LEGEND
    S- STANDARD 9 GAUGE LONGITUDINAL & CROSS WIRES
    H- HEAVY 5 mm LONGITUDINAL WIRES, 9 GAUGE CROSS WIRES
    .5 L- LADDER TYPE REINFORCEMENT
    T- TRUSS TYPE REINFORCEMENT
    .6 2- 2 LONGITUDINAL WIRES
    .7 SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR TYING MASONRY TO BACK UP WALLS.
    SPECIAL WALLS - SEE NOTES ON PLANS FOR ADDITIONAL REINFORCING AND GROUTING OTHER THAN INDICATED ABOVE
    REINFORCE CELLS @ END OF WALLS AT INTERSECTING WALLS & BESIDE OPENINGS.
    .8 GROUT MASONRY SOLID BELOW BEARING BASE PLATES FOR 500mm MIN.
    .9 PROVIDE A CONCRETE BOND BEAM COURSE c/w 1-20M CONT. USING LOW WEB BLOCKS AT THE TOP OF WALLS AND AT EACH FLOOR LEVEL U/N. GROUT COURSE SOLID.
    .10 PROVIDE 1-20M CORNER BAR (750 BEND x 750 BEND) AT AT CONCRETE BOND BEAM COURSES @ BLOCK WALL INTERSECTIONS.
    .11 PROVIDE "CLEAN OUTS" AT BOTTOM OF CELLS TO BE GROUTED TO ENSURE PROPER LAP LENGTH AND THAT CELL IS FILLED SOLIDLY. MAXIMUM GROUT LIFT IS 3 meters. GROUT TO HAVE 250mm SLUMP
    .12 EMBEDMENT OF MASONRY DOWELS IN CONCRETE STRUCTURE BELOW CONCRETE BLOCK WALLS TO BE AS FOLLOWS:
    15M DOWELS = 600 mm EMBEDMENT - 1300 Lg. DOWEL
    20M DOWELS = 800 mm EMBEDMENT - 1700 Lg. DOWEL
    .13 BLOCK CONTROL JOINT SPACED AT 9000 mm MAXIMUM VENEER CONTROL JOINT SPACED AT 12000 mm MAXIMUM COORDINATE LOCATION OF JOINTS WITH ARCHITECT & ENGINEER
15. LEGEND

B	=	BOTTOM
B1	=	BOTTOM LOWER LAYER
B2	=	BOTTOM UPPER LAYER
BLL	=	BOTTOM LOWER LAYER
BBP1	=	BEAM (OR OWSJ) BEARING PLATE NUMBER
BP1	=	BASE PLATE NUMBER
BUL	=	BOTTOM UPPER LAYER
C1	=	CONCRETE COLUMN NUMBER
CONT	=	CONTINUOUS
DP	=	DEPTH
DWL	=	DOWELS
EF	=	EACH FACE
EL	=	ELEVATION
ES	=	EACH SIDE
EW	=	EACH WAY
F1	=	PAD FOOTING NUMBER
H	=	HORIZONTAL
(H)	=	HOOKED BAR
MP1	=	MASONRY PIER NUMBER
O/C	=	ON CENTER
P1	=	PIER NUMBER/PILE NUMBER
SC1	=	STEEL COLUMN NUMBER
T	=	TOP
T1	=	TOP UPPER LAYER
T2	=	TOP LOWER LAYER
TLL	=	TOP LOWER LAYER
TUL	=	TOP UPPER LAYER
U/N	=	UNLESS NOTED OTHERWISE
V	=	VERTICAL
WF1	=	WALL FOOTING NUMBER

WIND UPLIFT (REF FIG I-9 NBC 2010 STRUCTURAL COMMENTARY I)

PNET = 1.4 (pe-pi) - 0.9 D
Pe = Iw q Ce Cp Cg
Pi = Iw q Ce Cpi Cgi
z = 2.0 m

Pf = 1.4 Pw NET - 0.9 Pd
Pw NET = Pe - Pi
Pw NET INTERIOR = 0.82 kPa
Pw NET PERIMETER = 1.07 kPa

DESIGN SNOW LOAD PARAMETERS
ARNPRIOR, ONTARIO, CANADA
S = Is [Ss(CbCwCsCa)+Sr]
Ss = 2.5 kPa
Sr = 0.4 kPa
Is = 1.10
S = 1.0 [2.5(0.8x1.0x1.0x1.0)+0.4]
S = 2.40 kPa

WIND

(2012 OBC 4.1.7, 2010 NBC COMMENTARY FIGURE I-7 TO I-9)
P= Iw q Ce Cp Cg

q = 0.37 kPa
Iw (uls) = 1.0 Iw (sls) = 0.75
Ce = VARIES FROM 0.9 TO 1.4
CpCg = 1.3 OR 1.95

	N.S (↑)	E.W(↔)	UNITS
VBASE	96.20	73.50	KN
MBASE	481	367	KN.m

NORTH FOR THE PURPOSES OF THIS DATA IS AT THE TOP SIDE OF ALL PLANS IN THIS SET OF DRAWINGS

DESIGN & DETAILING CRITERIA FOR SUPPLIERS

1. CURTAIN WALLS
   SUPPORTS FOR CURTAIN WALLS ARE TO BE DESIGNED AND DETAILED BY CURTAIN WALL SUPPLIER. SHOP DRAWINGS ARE TO BE SUBMITTED TO DESIGN TEAM FOR REVIEW. SHOP DRAWINGS ARE TO BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO. ALL SUPPORTS ARE TO BE INSPECTED DURING CONSTRUCTION BY THE SUPPORT DESIGN ENGINEER.
2. STRUCTURAL STEEL CONNECTIONS
   STRUCTURAL STEEL CONNECTIONS ARE TO BE DESIGNED AND DETAILED BY STRUCTURAL STEEL SUPPLIER. SHOP DRAWINGS ARE TO BE SUBMITTED TO DESIGN TEAM FOR REVIEW. SHOP DRAWINGS ARE TO BE STAMPED AND SIGNED FOR CONNECTIONS ONLY' BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO. INSPECTION OF WELDS, CONNECTIONS & INSTALLATION IS TO BE UNDERTAKEN BY A 3RD PARTY, CERTIFIED INSPECTION SERVICE.
3. COLD FORMED STEEL STUDS & JOISTS
   STEEL STUDS & JOISTS ARE TO BE DESIGNED AND DETAILED BY STEEL STUDS & JOISTS SUPPLIER. SHOP DRAWINGS ARE TO BE SUBMITTED TO DESIGN TEAM FOR REVIEW. SHOP DRAWINGS ARE TO BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO. ALL STEEL STUD & JOIST WORK IS TO BE INSPECTED DURING CONSTRUCTION BY THE STEEL STUD & JOIST DESIGN ENGINEER.
4. MISCELLANEOUS METALS & STEEL STAIRS
   MISC METALS & STEEL STAIRS ARE TO BE DESIGNED AND DETAILED BY MISC METALS & STEEL STAIRS SUPPLIER. SHOP DRAWINGS ARE TO BE SUBMITTED TO DESIGN TEAM FOR REVIEW. SHOP DRAWINGS ARE TO BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO. ALL MISC METAL & STEEL STAIR WORK IS TO BE INSPECTED DURING CONSTRUCTION BY THE MISC METALS & STEEL STAIRS DESIGN ENGINEER.
5. GUARDS & HANDRAILS
   GUARDS & HANDRAILS ARE TO BE DESIGNED AND DETAILED BY STEEL SUPPLIER IN ACCORDANCE WITH THE CURRENT BUILDING CODE REQUIREMENTS. SHOP DRAWINGS ARE TO BE SUBMITTED TO DESIGN TEAM FOR REVIEW. SHOP DRAWINGS ARE TO BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO. ALL GUARDS & HANDRAIL WORK IS TO BE INSPECTED DURING CONSTRUCTION BY THE GUARD & HANDRAIL DESIGN ENGINEER.
6. SEISMIC RESTRAINT OF MECH'L EQUIPMENT & PIPING
   SEISMIC RESTRAINT OF MECH'L EQUIPMENT & PIPING TO BE DETAILED BY MECH'L EQUIPMENT & PIPING SUPPLIER OR CONTRACTOR. SHOP DRAWINGS ARE TO BE SUBMITTED TO DESIGN TEAM FOR REVIEW. SHOP DRAWINGS ARE TO BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO. ALL SEISMIC RESTRAINT INSTALLATIONS ARE TO BE INSPECTED DURING CONSTRUCTION BY THE DESIGN ENGINEER OF RECORD
7. SEISMIC RESTRAINT OF SUSPENDED CEILINGS
   SEISMIC RESTRAINT OF SUSPENDED CEILINGS TO BE DETAILED BY CEILING SUPPLIER OR CONTRACTOR. SHOP DRAWINGS ARE TO BE SUBMITTED TO DESIGN TEAM FOR REVIEW. SHOP DRAWINGS ARE TO BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO. ALL SEISMIC RESTRAINT INSTALLATIONS ARE TO BE INSPECTED DURING CONSTRUCTION BY THE DESIGN ENGINEER OF RECORD
8. CONCRETE BLOCK MASONRY WALLS-CONSTRUCTION BRACING
   ALL LOAD BEARING CONCRETE BLOCK MASONRY WALLS ARE TO BE LATERALLY BRACED DURING CONSTRUCTION UNTIL STRUCTURE AND DIAPHRAGM IS CONSTRUCTED ON WALL. ALL NON LOAD BEARING CONCRETE BLOCK MASONRY WALLS ARE TO BE LATERALLY BRACED DURING CONSTRUCTION UNTIL PERMANENT LATERAL BRACING IS INSTALLED AS PER TYPICAL DETAILS AND OR SECTIONS.
   LATERAL CONSTRUCTION BRACING DRAWINGS ARE TO BE SUBMITTED TO DESIGN TEAM FOR REVIEW. SHOP DRAWINGS ARE TO BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO.
9. GLULAM CONNECTIONS AND HOLD DOWNS
   GLULAM CONNECTIONS AND HOLD DOWNS ARE TO BE DESIGNED AND DETAILED BY GLULAM SUPPLIER. SHOP DRAWINGS ARE TO BE SUBMITTED TO DESIGN TEAM FOR REVIEW. SHOP DRAWINGS ARE TO BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO.
10. TEMPORARY SHORING (FOR DEMOLITION AND/OR CONSTRUCTION)
    TEMPORARY SHORING FOR FOR THE PURPOSES OF DEMOLITION AND/OR CONSTRUCTION IS TO BE DESIGNED & DETAILED BY A PROFESSIONAL ENGINEER LICENSED IN ONTARIO. SHOP DRAWINGS ARE TO BE SUBMITTED TO THE DESIGN TEAM FOR REVIEW. SHOP DRAWINGS ARE TO BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO. PERMIT REVIEW OF TEMPORARY SHORING BY CUNLIFFE & ASSOCIATES PRIOR TO COMMENCEMENT OF CONSTRUCTION AND/OR DEMOLITION AND ALSO PRIOR TO REMOVAL OF TEMPORARY SHORING.
11. ROOF SCREEN
    ROOF SCREEN TO BE DETAILED BY SCREEN SUPPLIER. SHOP DRAWINGS ARE TO BE SUBMITTED TO DESIGN TEAM FOR REVIEW. SHOP DRAWINGS ARE TO BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO. ALL SCREEN INSTALLATIONS ARE TO BE INSPECTED DURING CONSTRUCTION BY THE DESIGN ENGINEER OF RECORD

NOTE:
INSPECTION REPORTS CREATED AS A RESULT OF THE ABOVE NOTED WORK MUST BE SUBMITTED TO THE CONSTRUCTION MANAGER. CONSTRUCTION MANAGER IS TO PROVIDE COPIES TO THE CONSULTANTS.

7	ISSUE FOR TENDER	OCT 11, 2017
6	ISSUE FOR TENDER REVIEW	OCT 06, 2017
5	ISSUE FOR TENDER	AUG 24, 2017
4	ISSUE FOR BUILDING PERMIT	AUG 18, 2017

No.	REVISION	DATE

1. THE CONTRACTOR IS RESPONSIBLE FOR CHECKING AND VERIFYING ALL DIMENSIONS. ANY DISCREPANCY SHALL BE REPORTED TO THE ENGINEER
2. THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL MATERIAL RELEVANT TO THE PROJECT
3. ADDITIONAL INFORMATION MAY BE ISSUED FOR CLARIFICATION TO ASSIST PROPER EXECUTION OF WORK. SUCH DRAWINGS WILL HAVE THE SAME MEANING AND INTENT AS IF THEY WERE INCLUDED WITH THE DRAWINGS IN THE CONTRACT DOCUMENTS
4. DO NOT SCALE DRAWINGS

PROJECT
1632 McNAB / BRAESIDE TOWN HALL

ARCHITECT
+VG ARCHITECTS

DRAWING
GENERAL NOTES

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ENGINEERS SEAL

SCALE 1 : 100

LICENCE OF AUTHORIZATION OF PROFESSIONAL ENGINEER OCT 11, 2017 J.C. CUFF

DRAWN	REVIEWED
PMD	JC

PROJECT No.	SHEET No.
17-052	S01

REVISION No. 7