1.0 GENERAL

1.1 STRUCTURAL DESIGN IN ACCORDANCE WITH THE ONTARIO BUILDING CODE 2012 AND THE USER'S GUIDE – NBC 2010 STRUCTURAL COMMENTARIES (PART 4 OF DIVISION B).

1.2 PERFORM ALL WORK TO THE REQUIREMENTS OF THE ONTARIO BUILDING CODE 2012. OBSERVE ALL LOCAL AND PROVINCIAL REGULATORY REQUIREMENTS AND EXECUTE ALL WORK TO THE REQUIREMENTS OF THE APPLICABLE CSA STANDARDS. ALL WORKMANSHIP TO BE REPRESENTATIVE OF THE HIGHEST INDUSTRY STANDARD.

1.3 COMPLY WITH LOCAL, PROVINCIAL, AND FEDERAL ENVIRONMENTAL REGULATIONS WHEN PERFORMING ALL WORK. COMPLY WITH ALL REQUIREMENTS OF THE OCCUPATIONAL HEALTH AND SAFETY ACT.

1.4 READ THESE DRAWINGS IN CONJUNCTION WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND CIVIL DRAWINGS. COORDINATE THE REQUIREMENTS OF THESE TRADES WITH THE STRUCTURAL WORK AND PROVIDE FOR OPENINGS, SLEEVES, DUCTS, ETC. IN THE CASE OF DISCREPANCIES, NOTIFY THE CONSULTANT IMMEDIATELY FOR CLARIFICATION. CUTTING OF REBAR IS NOT PERMITTED WITHOUT WRITTEN APPROVAL OF THE CONSULTANT. ALL CONCRETE ELEMENTS TO BE DRILLED OR CORED MUST BE SCANNED TO LOCATE EMBEDDED REBAR.

1.5 CONFIRM ALL DIMENSIONS, ELEVATIONS, GRADES AND EXISTING CONDITIONS PRIOR TO COMMENCING THE WORK AND REPORT ANY DISCREPANCIES TO THE CONSULTANT. DIMENSIONS ARE BASED ON ORIGINAL DESIGN DRAWINGS AND FIELD MEASURMENTS AND ARE NOT WARRANTED FOR ACCURACY.

1.6 PROPRIETARY SYSTEMS ARE TO BE INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.

1.7 INSTALL TEMPORARY HOARDING AND SHORING, AS REQUIRED TO PROTECT WORKERS AND OCCUPANTS OF THE SITE. MAINTAIN EXITS AT ALL TIMES. RESTORE DAMAGED CONSTRUCTION TO THE SATISFACTION OF THE CONSULTANT.

1.8 DO NOT SCALE THE DRAWINGS. DRAWING UNITS ARE IMPERIAL AND REFERENCE DIMENSIONS ARE IN INCHES, ELEVATION DIMENSIONS ARE IN FEET UNLESS NOTED OTHERWISE.

2.0 FOUNDATIONS AND BACKFILL

2.1 ALL FOUNDATION CONSTRUCTION TO BE IN ACCORDANCE WITH THE RECOMMENDATIONS AND INSTRUCTIONS OUTLINED IN THE GEOTECHNICAL REPORT AND ADDENDUM NO.1 PREPARED BY DST CONSULTING ENGINEERS.

2.2 ALL FOUNDATIONS SHALL BEAR ON NATIVE UNDISTURBED SOIL AT MINIMUM DEPTH OF 5' BELOW GRADE. THE FOLLOWING TABLE OF BEARING VALUES HAS BEEN USED IN THE DESIGN OF SQUARE FOOTINGS:

Depth (m)	Width of Footing (B) (m)	ULS Resistance (kPa)	SLS Resistance (kPa)
1.5	1.0	435	430
	1.5	445	280
	2.0	455	210
	2.5	465	160
1.8	2	540	210
	2.5	550	175
	3	560	150
	3.5	570	130
2.1	2.5	630	195
	3	640	165
2.4	3	725	175
3.2	1.5	910	375
4.7	1.5	1320	305

THE FOLLOWING TABLE HAS BEEN USED IN THE DESIGN OF STRIP FOOTINGS:

Depth (m)	Width of Footing (B) (m)	ULS Resistance (kPa)	SLS Resistance (kPa)
1.5	0.5	275	275
	1.0	295	190
	1.5	315	135

2.3 ENGINEERING FILL TO CONSIST OF GRANULAR 'B' TYPE I COMPACTED TO 98% SPMDD FROM 18'' BELOW UNDERSIDE OF SLAB ON GRADE TO 6'' BELOW SLAB ON GRADE PLACED IN 8'' MAXIMUM LIFTS. PLACE 6'' GRANULAR 'A' COMPACTED AT 100% SPMDD DIRECTLY UNDER SLAB ON GRADE. PROVIDE 12'' GRANULAR 'A' AROUND ALL PIPES AND MECHANICAL EQUIPMENT IN ENGINEERED FILL. COARSE MATERIAL (LARGER THAN 1'' DIAMETER) IS TO BE AVOIDED. SEE STANDARD DETAILS (S05).

2.4 LOCALLY REMOVE SLAB ON GRADE AS REQUIRED TO EXPOSE EXSISTING FOUNDATION. CONTACT CONSULTANT TO REVIEW EXISTNG CONDITIONS PRIOR TO PLACING FOOTING ALONG EXISTING WALLS AND ADJACENT TO EXISTING FOOTINGS.

2.5 PRIOR TO BACKFILLING AGAINST BASEMENT WALLS, GROUND FLOOR SLABS MUST BE PLACED AND MEET SPECIFIED STRENGTH OR TEMPORARY BRACING MUST BE PROVIDED.

3.0 WINTER PROTECTION

3.1 PROTECT ALL EXCAVATIONS, TEMPORARY WORKS, EXISTING/NEW STRUCTURES AND TANKS FROM FROST ACTION DURING CONSTRUCTION.

3.2 PROVIDE TEMPORARY HEAT, INSULATION MATERIALS OR OTHER MEANS AS REQUIRED TO PROTECT FOUNDATIONS FROM FREEZING.

4.0 CONCRETE

4.1 CONCRETE TO CAN/CSA A23.1-14 "CONCRETE MATERIALS AND METHODS OF CONCRETE CONSTRUCTION/ METHODS OF TEST AND STANDARD PRACTICES FOR CONCRETE". CONCRETE DESIGN TO CAN/CSA A23.3-04 (R2010) "DESIGN OF CONCRETE STRUCTURES".

4.2 CONCRETE MIXES AS FOLLOWS:

-MIX #1 INTERIOR CONCRETE STAIRS, SLAB ON GRADE, FOUNDATION WALLS, PIERS AND STRUCTURAL SLABS NOT EXPOSED TO FREEZING OR THAWING:
- CLASS OF EXPOSURE 'N'
- 30 MPa COMPRESSIVE STRENGTH AT 28 DAYS

-MIX #2 ALL EXTERIOR STAIRS, SLAB ON GRADE AND CONCRETE WALKS, PERIMETER FOUNDATION WALLS, PIERS;
- CLASS OF EXPOSURE 'C-1'
- 35 MPa COMPRESSIVE STRENGTH AT 28 DAYS
- AIR CONTENT IS 5 TO 8%

4.3 CONTRACTOR TO NOTIFY CONSULTANT A MINIMUM OF 48 HOURS PRIOR TO ANY CONCRETE PLACEMENT

4.4 AT LOCATIONS BELOW GRADE WHERE NEW CONCRETE IS PLACED AGAINST EXISTING CONCRETE, UNLESS NOTED OTHERWISE PROVIDE DOWELS INTO EXISTING TO MATCH THE SPACING OF THE CONNECTING WALL/SLAB/FOOTING. PROVIDE MINIMUM OF 6" EMBEDMENT INTO EXISTING WITH HILTI HIT HY 200 w/ SAFESET SYSTEM.

4.5 PREVIOUSLY PLACED CONCRETE AT CONSTRUCTION JOINTS TO BE WIRE BRUSHED, CLEANED AND MOISTENED IMMEDIATELY PRIOR TO PLACING FRESH CONCRETE. INTENTIONALLY ROUGHEN CONCRETE AT CONSTRUCTION JOINTS TO ¼" AMPLITUDE TO EXPOSE AGGREGATES AND PROVIDE INTERLOCK BETWEEN CONCRETE PLACEMENTS. WHERE CASTING NEW CONCRETE AGAINST EXISTING, ROUGHEN CONCRETE TO 5mm AMPLITUDE TO EXPOSE AGGREGATE AND PROVIDE INTERLOCK BETWEEN CONCRETE SURFACES.

4.6 CURE CONCRETE TO CSA A23.1/A23.2. TAKE APPROPRIATE PRECAUTIONS FOR HOT AND COLD WEATHER WORK.

4.7 THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN, FABRICATION, INSPECTION AND DISMANTLING OF FALSEWORK AND FORMWORK IN ACCORDANCE WITH CAN/CSA A23.1/A23.2; CAN/CSA A269.1 "FALSEWORK FOR CONSTRUCTION PURPOSES"; AND CAN/CSA A269.3 "FORMWORK FOR CONSTRUCTION PURPOSES". SUBMIT FALSEWORK AND FORMWORK DRAWINGS AND CALCULATIONS STAMPED AND SIGNED BY A QUALIFIED ENGINEER LICENSED IN THE PROVINCE OF ONTARIO, CANADA CLEARLY INDICATING MEMBER SIZES, RATE OF PLACEMENT AND LOADS.

4.8 ALL FALSEWORK AND FORMWORK SHALL BE LEFT IN PLACE UNTIL THE CONCRETE HAS SUFFICIENT STRENGTH TO SUPPORT THE SELF-WEIGHT OF THE CAST MEMBERS AND CONSTRUCTION LOADS. FORM REMOVAL REQUIRES WRITTEN APPROVAL FROM THE ENGINEER. THE MINIMUM TIME FORMS ARE TO BE LEFT IN PLACE FOR VERTICAL ELEMENTS (WALLS AND COLUMNS) SHALL BE 24 HOURS. THE MINIMUM TIME FALSEWORK AND FORMS SHALL BE LEFT IN PLACE FOR ALL SUSPENDED STRUCTURAL ELEMENTS IS 7 DAYS.

4.9 ANCHOR BOLTS ARE TO BE ASTM F1554 FOR ALL ANCHOR BOLTS AT COLUMNS IN BRACED BAYS UNLESS NOTED OTHERWISE.

5.0 FLOOR SLAB SUPPORTED ON GRADE

5.1 REINFORCEMENT IS TO BE CHAIRED OFF THE SUBGRADE PRIOR TO PLACING CONCRETE. APPLY VAPOR BARRIER, IN SPECIFIED LOCATIONS, PRIOR TO PLACING THE CONCRETE.

5.2 CONCRETE IS TO BE PLACED, SCREEDED AND FLOATED TO ENSURE A WELL COMPACTED, VOID-FREE SLAB. ALL AREAS, EXCEPT AS NOTED SLAB FINISH, TO BE CLASSIFIED AS VERY FLAT (WITHIN 3 mm (1/8 in.) OF A 3 m (10 ft) STRAIGHT EDGE). SLAB FINISH TO BE IN ACCORDANCE WITH CAN/CSA A23.1 - LATEST EDITION, NON-SLIP STEEL TROWEL FINISH, UNLESS NOTED OTHERWISE. SLOPE TO DRAINS AS NOTED ON DRAWINGS.

5.3 ALL FLOOR SLABS ARE TO BE SEALED. REFER TO ARCHITECTURAL SPECIFICATIONS FOR SPECIAL FINISHING, ETC.

5.4 PROVIDE SAWCUTS AT MAX 3 m (10 ft) SPACING OR AS NOTED. PROVIDE SAW CUT AT CONSTRUCTION JOINT LAYOUT FOR REVIEW.

6.0 REINFORCING

6.1 ALL REINFORCING STEEL TO BE NEW, DEFORMED BARS CONFORMING TO CSA G30.18 GRADE 400 W.

6.2 CONCRETE CLEAR COVER TO PRIMARY REINFORCING:

ELEMENT AND EXPOSURE:	COVER(mm)	COVER (in)
ALL WALLS	50 mm	2 in
SLABS ON GRADE (TOP)	50 mm	2 in
SLABS ON GRADE (BOTTOM)	75 mm	3 in
COVER AGAINST SOIL OR ROCK	75 mm	3 in
COVER EXPOSED TO SOIL, ROCK OR WEATHER	50 mm	2 in

6.3 PROVIDE CLASS 'B' TENSION LAP SPLICES FOR ALL SPLICE LOCATIONS AND ALL BARS INDICATED AS BEING CONTINUOUS.

6.4 ALL REINFORCING STEEL TO BE CHAIRED AND SECURELY TIED IN PLACE USING STANDARD TIES AND CHAIRS TO THE REQUIRED COVER FOR EXPOSED CONCRETE, CHAIRS AND BOLSTERS TO BE PLASTIC TIPPED OR STAINLESS STEEL.

6.5 UNLESS OTHERWISE NOTED, PROVIDE MATCHING CORNER BARS AND DOWELS AT ALL WALL INTERSECTIONS, WALL FOOTING INTERSECTIONS, AND COLUMN FOOTING INTERSECTIONS.

6.6 SUBMIT REINFORCEMENT SHOP DRAWINGS DETAILING ALL REINFORCEMENT IN ACCORDANCE WITH RSIC MANUAL OF STANDARD PRACTICE.

7.0 STRUCTURAL STEEL

7.1 STRUCTURAL STEEL DESIGN IN ACCORDANCE WITH CAN/CSA S16-09 - DESIGN OF STEEL STRUCTURES AND THE CANADIAN INSTITUTE OF STEEL HANDBOOK OF STEEL CONSTRUCTION, TENTH EDITION.

7.2 ALL STRUCTURAL STEEL AND MISCELLANEOUS METALS TO CONFORM TO:

- WIDE FLANGE (W) TO: CAN/CSA-G40.20/G40.21 GRADE 350W
- PLATES, ANGLES (L) AND CHANNELS (C) TO: CAN/CSA-G40.20/G40.21 GRADE 300W
- HOLLOW STRUCTURAL SECTIONS (HSS) TO: CAN/CSA-G40.20/G40.21 GRADE 350W, CLASS C
- METAL DECK TO BE GRADE 230MPA AND GALVANIZED AS NOTED ON THE PLAN TO ASTM A653. MINIMUM 3 SPAN DESIGN ASSUMED

7.3 HOT DIP GALVANIZING TO ASTM A123/A123M-09 – STANDARD SPECIFICATION FOR ZINC (HOT DIP GALVANIZED) COATINGS ON IRON AND STEEL PRODUCTS, 610g/m² MINIMUM

7.4 ALL BOLTED CONNECTIONS TO USE BOLTS IN ACCORDANCE WITH ASTM A325 - 10 STANDARD SPECIFICATION FOR STRUCTURAL BOLTS, STEEL, HEAT TREATED, 120/105 ksi MINIMUM TENSILE STRENGTH, MINIMUM TWO 19mm (3/4") DIAMETER BOLTS PER CONNECTION. ALL HIGH STRENGTH BOLTED CONNECTIONS SHALL BE INSTALLED TO A SNUG-TIGHTENED CONDITION PRIOR TO INSPECTION. THE CONTRACTOR IS NOT TO PRE-TENSION BOLTS UNTIL SNUG TIGHT CONDITION HAS BEEN VERIFIED. THE STRUCTURAL STEEL INSPECTOR SHALL BE PRESENT DURING PRE-TENSIONING TO ASCERTAIN THAT THE PROPER PROCEDURES ARE EMPLOYED.

7.5 DESIGN AND DETAIL CONNECTIONS IN ACCORDANCE WITH REQUIREMENTS OF CAN/CSA S16-09 TO RESIST FORCES, MOMENTS, SHEARS AND ALLOW FOR MOVEMENTS INDICATED. SELECT OR DESIGN CONNECTIONS TO SUPPORT REACTIONS FROM MAXIMUM UNIFORMLY DISTRIBUTED LOAD THAT CAN BE SAFELY SUPPORTED BY BEAM IN BENDING, PROVIDED NO POINT LOADS ACT ON THE BEAM AND AS A MINIMUM 75% OF THE SHEAR CAPACITY OF THE MEMBER, WHICHEVER IS GREATER. SUBMIT DRAWINGS AND DESIGN CALCULATIONS, SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF ONTARIO, IN ACCORDANCE WITH PROJECT SPECIFICATIONS

7.6 MISCELLANEOUS MATERIALS AND ACCESSORIES ASSOCIATED WITH GOOD PRACTICE THAT ARE NOT SHOWN SHALL BE PROVIDED.

7.7 DRIFT PINS SHALL NOT BE USED TO ENLARGE MISALIGNED OR UNFAIR BOLT HOLES. HOLES THAT REQUIRE ENLARGING SHALL BE REAMED.

7.8 THE CONTRACTOR IS RESPONSIBLE FOR ALL TEMPORARY SHORING, BRACING AND SUPPORTS TO ADEQUATELY MAINTAIN THE PARTIALLY ERECTED STEEL IN PLACE DURING THE WORK. SUBMIT ERECTION DRAWINGS AND CALCULATIONS STAMPED AND SIGNED BY A QUALIFIED PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO INDICATING SEQUENCE OF ERECTION, ALL BRACING AND LOADS.

7.9 WELDING IN ACCORDANCE WITH CSA W59. ELECTRODES TO BE E49XX. ALL WELDS TO BE CONTINUOUS UNLESS NOTED OTHERWISE. THE MINIMUM FILLET WELD UNLESS NOTED OTHERWISE IS 6mm.

7.10 PRIOR TO BEGINNING ANY STRUCTURAL WELDING, SUBMIT PHOTOCOPIES OF ALL CWB WELDING CERTIFICATES OR WELDERS. QUALIFICATIONS FOR CERTIFICATES PROVIDED SHALL MATCH PROPOSED WELDS TO BE USED IN CONNECTIONS.

7.11 THE CONTRACTOR IS TO SUBMIT STRUCTURAL STEEL SHOP DRAWINGS PREPARED IN ACCORDANCE WITH CSA S16-09 PRIOR TO FABRICATION. SHOP DRAWINGS ARE TO BE SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF ONTARIO.

7.12 PERIMETER ANGLES, PLATES AND BACK TO BACK ANGLES SPECIFIED ON ROOF PLANS ARE DIAPHRAGM CHORD MEMBERS OF DIAPHRAGM COLLECTOR MEMBERS. ALL SPLICES IN THESE MEMBERS ARE TO BE FULL SPLICE CONNECTIONS CAPABLE OF DEVELOPING THE FULL TENSILE CAPACITY OF THE MEMBER.

7.13 ALL EXPOSED STEEL IS TO BE HOT DIPPED GALVANIZED TO ASTM A123/A123M-09-STANDARD SPECIFICATION FOR ZINC (HOT DIPPED GALVANIZED) COATINGS ON IRON AND STEEL PRODUCTS.

7.14 BRACED BAY CONNECTION LOADS PROVIDED ON BRACED BAY ELEVATIONS INCLUDE THE Ro AMPLIFICATION REQIURED IN CAN/CSA S16 CLAUSE 27.10.

8.0 STEEL JOIST FRAMING

8.1 ALL JOIST TOP CHORDS ARE TO BE MINIMUM OF ¼'' IN THICKNESS FOR FASTENING OF STEEL DECK DIAPHRAGM TO JOISTS WITH HILTI X-ENP-19 L15 FASTENERS.

8.2 DESIGN OPEN WEB STEEL JOISTS IN ACCORDANCE WITH CAN/CSA S16-09 – LIMIT STATES DESIGN OF STEEL STRUCTURES FOR DEAD AND LIVE SERVICE LOADS AND POINT LOADS IDENTIFIED ON THE PLANS.

8.3 OWSJ DEFLECTIONS NOT TO EXCEED L/360 FOR LIVE LOAD DEFLECTIONS AND L/240 FOR TOTAL LOAD DEFLECTIONS.

8.4 REQUIRES OWSJ SHOE DEPTH AS NOTED ON THE PLANS

8.5 PROVIDE OWSJ BRIDGING TO MEET CAN/CSA S16-09 AND THE REQUIREMENTS OF THE OWSJ DESIGN. BRIDGING SHOWN ON THE PLANS IS CONSIDERED TO BE THE MINIMUM BRIDGING REQUIREMENTS AND THE OWSJ MANUFACTURER IS TO DETERMINE BRIDGING REQUIREMENTS TO SAFETLY SUPPORT LOADING INDICATED.

8.6 DEPTH OF OWSJ TOP CHORD EXTENSIONS NOT TO EXCEED OWSJ SHOE DEPTH.

9.0 MASONRY

9.1 MASONRY WORK TO BE PERFORMED IN ACCORDANCE WITH CSA S304.1 - LATEST EDITION AND CSA A371, "MASONRY CONSTRUCTION FOR BUILDINGS."

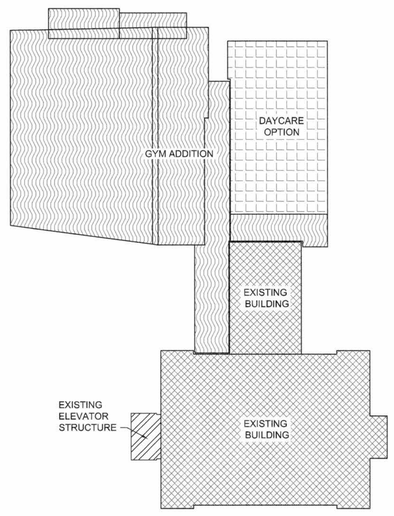
9.2 CONCRETE BLOCKS TO BE TYPE H/15/A/M, UNLESS NOTED OTHERWISE AND TO CONFORM TO CSA A165 - LATEST EDITION.

9.3 COARSE AGGREGATE PEA GRAVEL GROUT TO BE MIXED TO PROPORTION SPECIFICATIONS, TO CSA A179-04.

9.4 FOR VERTICAL REINFORCING IN MASONRY WALLS REFER TO SCHEDULE ON DRAWINGS. PROVIDE TWO VERTICALS (ONE IN EACH CORE) AT ALL WALL ENDS AND ON EACH SIDE OF OPENINGS AND FIVE VERTICALS AT EACH CORNER AND AT 'T' INTERSECTIONS UNLESS NOTED OTHERWISE ON DRAWINGS.

9.5 FILL ALL CELLS CONTAINING VERTICAL OR HORIZONTAL REINFORCING BARS, CAST-IN OR DRILLED-IN ANCHORS, WITH COARSE AGGREGRATE PEA-GRAVEL GROUT.

9.6 PROVIDE MASONRY LINTELS AS NOTED AND AS REQUIRED IN NEW CONSTRUCTION. EXTEND ALL LINTEL REINFORCING AND SOLID GROUT FILL 200 mm (8") PAST EDGE OF OPENING ON BOTH SIDES UNLESS NOTED OTHERWISE.



KEY PLAN

ABBREVIATIONS

BLDG	BUILDING
BM	BEAM
BOT	BOTTOM
B.P.	BASE PLATE
crs.	ON CENTRE
CJ	CONSTRUCTION JOINT
CL	CENTRE LINE
CANT	CANTILEVER
COL	COLUMN
CONC	CONCRETE
CONT.	CONTINUOUS
C/W	COMPLETE WITH
DIA or Ø	DIAMETER
DP	DEEP
DWG(S)	DRAWING(S)
B.A.	EACH FACE
EL.	ELEVATION
ELEV	ELEVATION
EQ	EQUAL
E.W.	EACH WAY
EXST	EXISTING
FDN	FOUNDATION
FTG	FOOTING
Ga	GAUGE
GALV	GALVANIZED
HORIZ	HORIZONTAL
HP	HIGH POINT
LB	LOAD BEARING
LL	LIVE LOAD
LLV	LONG LEG VERTICAL
LLH	LONG LEG HORIZONTAL
LP	LOW POINT
MAX	MAXIMUM
MECH	MECHANICAL
MIN	MINIMUM
MISC	MISCELLANEOUS
N/A	NOT APPLICABLE
NLB	NON LOAD BEARING
NTS	NOT TO SCALE
OWSJ	OPEN WEB STEEL JOIST
R	RADIUS
RD	ROOF DRAIN
REINF	REINFORCING / REINFORCE
REQ'D	REQUIRED
REV	REVISION or REVISED
RO	ROUGH OPENING
R/W	REINFORCED WITH
SECT	SECTION
SPDD	STANDARD PROCTOR DRY DENSITY
SS	STAINLESS STEEL
STD	STANDARD
SQ	SQUARE
TJ	TIE JOIST
T.O.	TOP OF
TOC	TOP OF CONCRETE
TOS	TOP OF STEEL / TOP OF SLAB
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
U/S	UNDERSIDE
VERT	VERTICAL
w/	WITH
w/o	WITHOUT

No.		
1	FOR TENDER	01/06/18
No.	ISSUE / REVISION	DD/MM/YY

VERIFY SHEET SIZE AND SCALES. BAR TO THE RIGHT IS 1" IF THIS IS A FULL SIZE DRAWING. 0 1"

SCALE: ----

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PROJECT NORTH: LICENSED PROFESSIONAL ENGINEER M. J. BURTT 100111392 PROVINCE OF ONTARIO

PROJECT:
MAISON DE LA FRANCOPHONIE D'OTTAWA
2720 RICHMOND ROAD, OTTAWA

DRAWING:
GENERAL NOTES

DESIGN: SDB/VR	
DRAWN: JPS	DRAWING #: S01
CHECKED: MJB	
JLR #: 27672-001	

File Location: R:\27000\27672-000.1 - capeo - 2720 richmond road (cmk)\JLR DWG\Struct\27672 S01 S02.dwg

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