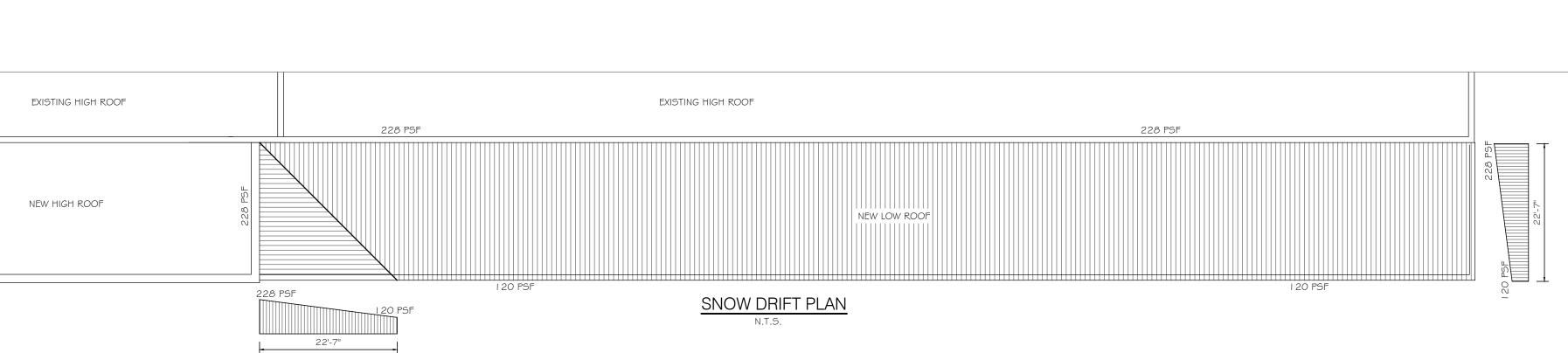
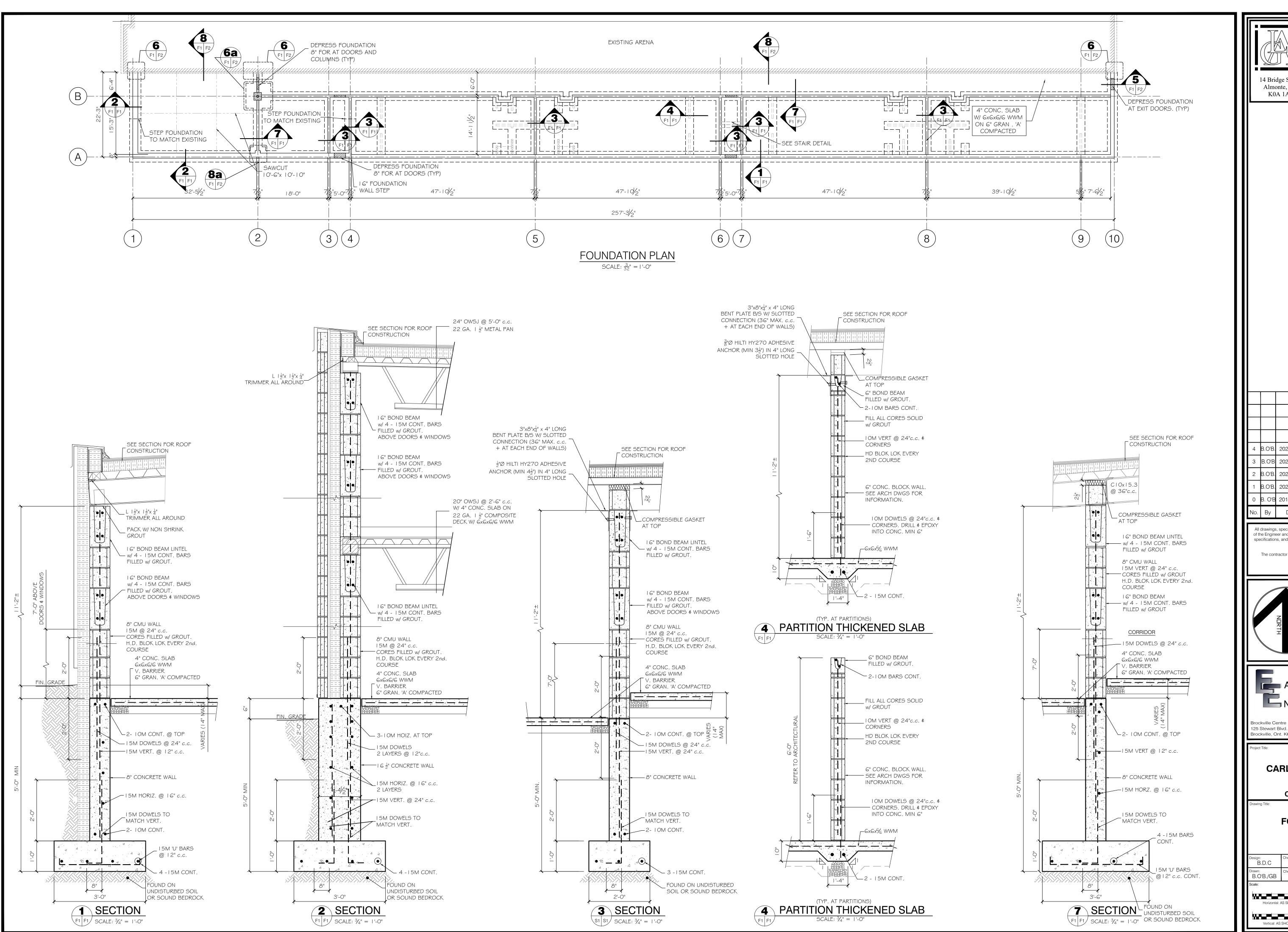
<u>ENERAL NOTES</u> OVERNING CODE - (ONTARIO BUILDING	CODE OF CANAD	A. 2012 ED	DITION:					MIN. REINFORCING AROUND OPENINGS LARG
DESIGN LOADS: (MATCH VERTICAL BARS. PROVIDE SUFFICIENT CHAIRS AND SUPPORT B
GROUND SNOW L	DAD:	Ss = 2.5 kPa Sr = 0.4 kPa							RUCTURAL STEEL:
ROOF:	DEAD LOAD	Is = 1.15 $= 25 psf$ $= 57 psf$	FER TO DRIF	FT LOADS				1.	FABRICATE AND ERECT STRUCTURAL STEEL 1 MATERIAL SPECS. FOR REVIEW PRIOR TO FAB
SECOND FLOOR L	SNOW LOAD <u>OADS:</u> DEAD LOAD: 75 psf LIVE LOAD: 50 psf	= 57 psf							REGISTERED IN ONTARIO, FOR THOSE CONNEC SITE TO SATISFY HIMSELF THAT THESE CONNI ENGINEER SHALL PROVIDE A LETTER TO THE MODIFICATIONS, MADE TO THIS DESIGN.
MAIN FLOOR LOA	100 psf	(MEETING ROOM)	AND EXITS	8)				2.	PROVIDE STRUCTURAL STEEL TO CSA G40.21- WIDE FLANGE BEAMS AND COLUMNS CHANNELS
WIND LOAD:	DS. DEAD LOAD LIVE LOAD q $(1/50) = 0.41$ kPa	= 60 psf = 100 psf (IN	CLUDING I	EXITS)					HSS SECTIONS (CLASS H) STRUCTURAL BARS, PLATES & ANGLES MISCELLANEOUS STEEL PIPE COLUMNS
	$I_{W} = 1.15$	$S_{2}(1,0) = 0.002$	DC	A - 0 102	,			3.	PROVIDE ERECTION BOLTS TO ASTM A325, M
<u>SEISMIC:</u>	Sa (0.2)= 0.302 Sa (0.5)= 0.175 SITE CLASS: "D"	Sa (1.0)= 0.093 Sa (2.0)= 0.046		A= 0.192 = 1.3	2				WELD TO CASA W59-13 BY FABRICATORS CER MINIMUM WELDS FOR CONNECTIONS SHALL
							F THE 2012 ONTARIO BUILDING .25 PER ONTARIO BUILDING CODE.		ALL STUD ANCHORS AND DEFORMED BAR AN RECOMMENDATIONS.
DUNDATIONS - GEN THE FOUNDATION		D FOR AN ALLOW	ABLE BEA	.RING CA	APACITY (OF 10(KPa AS INDICATED IN THE GEO-TECHNICAL	7.	THE TOTAL BEAM LOAD CAPACITY AS LISTEI
THE SUITABILITY	.06 COMPLETED BY H OF BEARING MATER OT BE POURED INTO	RIALS. SOME ADJU	ISTMENTS	MAY BI			CAL ENGINEER SHALL BE THE SOLE JUDGE AS TO		SHOWN ON PLANS ARE SPECIFIED LOADS. SE UNLESS OTHERWISE NOTED, ALL CONNECTIO DOUBLE ANGLE OR TEE-TYPE CONNECTIONS
	NS SHALL BE DE-WA JE TO EXTREME WEA					EQUIR	ED TO PROTECT BEARING MATERIALS FROM		FOR CONNECTIONS NOT DESCRIBED ABOVE N SIMPLE CONNECTION AND DESIGN FOR THE S
						CK. FO	DOTING ELEVATIONS MAY NEED TO BE ADJUSTED.		SIMPLE BEAM TO COLUMN CONNECTIONS SH. CONNECTIONS SHALL BE DESIGNED TO DELIV
TIME OF INSPECT	ON, REINFORCEMEN	T MUST BE MINIM					TE. FORMWORK SHALL BE COMPLETE AT THE NSPECTION. PROVIDE MINIMUM 24 HOUR NOTICE		UNLESS OTHERWISE NOTED, CONNECTIONS A DIA. BOLTS).
5.1. FOOTING PLA		115:							BOLTED CONNECTIONS FOR DRAG STRUT LIN
.3. SLAB ON GRA	WALL PLACEMENT DE PLACEMENT			_				8.	ALL CONNECTIONS. TEMPORARY BRACING DURING CONSTRUCTION
							MULTANEOUSLY SO AS TO MAINTAIN A COMMON ROVED BY ENGINEER.	9.	RESPONSIBLE FOR SAFETY ON SITE. COORDINATE WITH MECHANICAL, PLUMBING
	R IS RESPONSIBLE TO BLE LIMITS (1:500).	PROVIDE ADEQU	ATE BRAC	ING ANI	D SUPPOR	T OF I	ORMWORK TO ENSURE WALLS ARE PLUMB		NO BURNING OF HOLES SHALL BE ALLOWED
II COMPACTED TO		GRANULAR FILL S	HALL BE C	COMPAC	CTED IN M	IAXIM	OTHER FILL ON SITE SHALL BE GRANULAR B TYPE UM 10" LIFTS. FILL NOT SPECIFIED ON THE MENT.	11.	FOR MISC. STEEL SUCH AS RAILINGS, AWNING ENGINEER SHALL CHECK SHOP DRAWINGS AI OTHER EFFECTS REQUIRED BY ONTARIO BUII SUB-CONTRACTOR'S RESPONSIBILITY. ALL RA
DNCRETE: DESIGN OF CONCL	ETE ELEMENTS SHA	LL CONFORM TO (CSA-A23.3-	14. PRO	VIDE CON	NCRET	E AND PERFORM WORK TO CSA A23.3-14.	12.	ALL VISUALLY EXPOSED SURFACES OR SURF PRIMER TO CISC/CPMA 1-73A (REFER TO ARCI
TEST CONCRETE I	N ACCORDANCE WIT	H CSA-A23.3-14.						13.	ANTI-RUST PAINT. PROVIDE NAIL HOLES (2 MIN.) IN MISC. STEEI
CONCRETE REQU	REMENTS: MIN. DESIGN ST	ſR.						14.	SEE NOTE "8" IN STEEL DECKING NOTES FOR I
LOCATION CONDITION	Mpa = TYPE (DA C = COMPRESSI F= FLEXURAL	AYS) <u>CEMEN</u>	$\underline{\mathbf{T}} \qquad \underline{\mathbf{AI}} \qquad \underline{\mathbf{AI}} \qquad \underline{\mathbf{(\%)}}$		AX AGG. (mm)	<u>EX</u>	<u>P.</u>	15.	CHECK MECHANICAL AND ARCHITECTURAL STRUCTURAL DRAWINGS FOR MECHANICAL CONTRACTOR MUST PROVIDE CONFIRMATIO DECK SHOP DRAWING APPROVAL STAGE OF
SLAB ON GRADI FOOTINGS	2 25C (28) 20C (28)	GU GU	< 3 < 3		20 20	N N		16.	THE STEEL STRUCTURE IS A NON-SELF-SUPPO ATTACHMENT TO THE WALL SYSTEM FOR ST
FOUNDATION W	ALLS 25C (28)	GU	4-7		20	F-2			REQUIRED FOR STABILITY AND FOR RESISTA PROVIDING THIS SUPPORT.
ALL CONCRETEWATER CEMENDO NOT USE AN	ED STRENGTH EXCEE TO BE NORMAL WEIG I RATIOS FOR EXPOSI Y ADMIXTURE CONT LAND CEMENT TO BE	GHT 2400 KG/CUBI SURE CLASSES AS I FAINING CHLORIDI	C METER PER TABLE E FOR CON	ES 7 - 9, 0	CAN/CSA-A	A23.1-		17.	THE FABRICATOR IS RESPONSIBLE FOR THE D AND ARE ONLY INTENDED TO SHOW THE REL INCORPORATED INTO FABRICATOR'S CONNEC FABRICATOR'S ENGINEER WITH THE ENGINEE "FOR DESIGN OF CONNECTIONS ONLY".
CONTROL JOINTS UNLESS OTHERW		E: SAWCUT AT LOC	CATIONS S	HOWN (ON DRAW	INGS I	BUT NOT EXCEEDING 15'-0" SPACING.	18.	REFER TO SCHEDULE ON DWG FOR NON-BEA SIZES AND LOCATIONS.
NO COLUMN OR V FOR OTHER COLU		BE REMOVED BEF	ORE CONC	RETE HA	AS REACH	HED 10	Mpa FOR ARCHITECTURAL CONCRETE AND 8 Mpa		PROVIDE HOLES IN STEEL MEMBER FOR ATTA
NO SLAB FORMS	OR BEAM FORMS SHA	LL BE REMOVED	BEFORE CO	ONCRET	E HAS RE	ACHE	D 17 Mpa.	20.	NO SPLICES IN COLUMNS AND BEAMS ARE AI ULTRASONICALLY TESTED OR EQUAL AND A
	NCRETE FOR STRIPPI DESIGN ENGINEER, 1		IINED BY I	FIELD-C	URED CYI	LINDE	RS. ALTERNATE METHODS, IF ACCEPTABLE TO		ALL LINTELS SHALL HAVE A MINIMUM OF 20 PROVIDE MASONRY ANCHORS IN CONFORMA
	E APPROVED BY THE						D D LOTH		ADJACENT TO MASONRY.
). BE RESPONSIBLE		I. MIX DESIGN SH	ALL BE PR	OPORTI	ONED WIT	TH DU	E CONSIDERATION TO EXTREME TEMPERATURES -	23.	STEEL BEAMS BEARING ON MASONRY SHALL 4") STRAP ANCHOR X 300mm (12") LONG WELE
	E ENGINEER PRIOR T		CTOR TO D	DETERMI	INE REQU	IREM	ENTS. ADMIXTURES AND ADDITIVES SHALL BE		
REINFORCING STI BARS TO BE WELI	– EEL: NEW DEFORMED						RS FOR CONCRETE REINFORCEMENT", GRADE 400R, ASTM A307. EPOXY COATED REBAR TO ASTM A775.		
	CONCRETE COVER OV	/ER REBAR AS FOI	LOWS:						
B) FORMED SUR	JRED AGAINST GROU FACES EXPOSED TO C	GROUND OR WEAT		75 mr 40 mr					
BEAMS	FACES NOT EXPOSED (TO STIRRUPS) INS (TO VERTICALS)	TO GROUND OR V	VEATHER:	40 mi 50 mi	m				
D) PARKING SUF	WALLS FACES AND EXTERIC			20 mr 40 mr	m				
	RK TO BE INSPECTED EEL ARE IN PLACE FO			Y THE EI	NGINEER	24 HO	URS IN ADVANCE. CONTRACTOR MUST ENSURE		
	E LENGTHS (UNLESS		,		STDENCT	тиς (~	n)		
	<u>25 Mpa</u>	<u>CLICE "CLASS B"</u> <u>30 Mpa</u>	35	Mpa		Mpa	<u>"'</u>		
10M 330 15M 480 20M 580	740 (940)	535 (790) 660 (865)	510 610	0 (460) 0 (660) 0 (815)	480 580) (430)) (610)) (760)			
			AND SHOU) (1220 EN HC) RIZONTAL SPLICE BARS ARE PLACED SUCH THAT		
		S NOTED ON DRA	·		CMBEDME E STRENG	GTH (r			
TOP BAR SPLICE I THERE IS MORE T REBAR EMBEDMI BAR COMPI SIZE <u>FOR CO</u>	ENT LENGTHS (UNLES RESSION EMBEDMEN NCRETE STRENGTH	<u>(mm)</u>	FOR CO			4	Mpa		
TOP BAR SPLICE I THERE IS MORE T REBAR EMBEDMI BAR COMPI SIZE <u>FOR CO</u> <u>20 Mpa</u>	RESSION EMBEDMEN	<u>(mm)</u> <u>25 N</u>	FOR CO Ipa <u>3</u>	<u>NCRETI 0 Mpa</u> 05 (380)	35 Mpa 80 (360)	_	0 (330)		
TOP BAR SPLICE I THERE IS MORE T REBAR EMBEDMI BAR COMPI SIZE FOR CO 20 Mpa 10M 250(330) 15M 360 (450) 20M 430 (535)	ESSION EMBEDMEN NCRETE STRENGTH 25 Mpa 30 Mpa 230 (280) 200 (250) 300 (400) 275 (360) 380 (485) 360 (450)	(mm) <u>25 M</u> 330 (460 (560 (FOR CO Ipa 3 (430) 36 (610) 42 (735) 5	<u>0 Mpa</u> 05 (380) 30 (530) 10 (660)	80 (360) 80 (510) 80 (610)) 51) 31	0 (330) 0 (480) 0 (585)		
TOP BAR SPLICE I THERE IS MORE T REBAR EMBEDMI BAR COMPI SIZE FOR CO 20 Mpa 10M 250(330) 15M 360 (450) 20M 430 (535) 25M 535 (710) 30M 635 (840)	ESSION EMBEDMEN NCRETE STRENGTH 25 Mpa 30 Mpa 230 (280) 200 (250) 300 (400) 275 (360)	(mm) 330 (460 (560 (915 (1070	FOR CO Ipa 3 (430) 30 (610) 4 (735) 5 (1170) 8	0 Mpa 05 (380) 30 (530) 10 (660) 40 (1070) 0 (1270)	80 (360) 80 (510) 80 (610) 760 (990) 15 (1170)) 50) 30) 60)) 10	0 (330) 0 (480) 0 (585) 0 (940) 0 (1120)		
TOP BAR SPLICE I THERE IS MORE T REBAR EMBEDMI BAR COMPI SIZE FOR CO 20 Mpa 10M 250(330) 15M 360 (450) 20M 430 (535) 25M 535 (710) 30M 635 (840) 35M 760 (990) TOP BAR DEVELO	RESSION EMBEDMEN NCRETE STRENGTH 25 Mpa 30 Mpa 230 (280) 200 (250) 300 (400) 275 (360) 380 (485) 360 (450) 485 (635) 450 (585) 585 (740) 535 (685) 685 (900) 635 (810)	(mm) 25 N 330 (460 (560 (915 (1070 1295 E DENOTED IN BR	FOR COI Ipa 30 (430) 30 (610) 4 (735) 5 (1170) 8 (1400) 90 (1675) 17 ACKETS A	0 Mpa 05 (380) 30 (530) 10 (660) 40 (1070) 0 (1270) 70 (1525) ND SHO	80 (360) 80 (510) 80 (610) 760 (990) 15 (1170) 900 (142)) 50) 30) 60)) 10)) 40 20) 10	0 (330) 0 (480) 0 (585) 0 (940) 0 (1120)		

ER THAN 300 mm: 2 - 15M EACH SIDE OF OPENING, EXTENDED 600 mm PAST CORNERS.	METAL DECK:
E IS POURED. TEMPLATES SHALL BE USED TO ENSURE CORRECT PLACEMENT OF DOWELS. DOWELS TO ARS TO MAINTAIN CONCRETE COVER AS SPECIFIED.	 DESIGN, FABRICATE, AND INSTALL STEEL DECK TO CSA S136-09 AND THE CSSBI 101M84 GRADE A UNLESS OTHERWISE NOTED. FOR ROOFS UNLESS OTHERWISE NOTED ON PLAN. MINIMUM COATING DESIGNATION ZF075 WIRE COAT FINISH. PROVIDE METAL GAUGE AND FASTENERS TO CARRY THE VERTICAL LOADS SPECIFIED ON THE DRAWINGS.
	2. DECKING PROFILE: AS NOTED ON DRAWING.
O CAN/CSA S16.1-14. SUBMIT TWO SETS OF PRINTS OF SHOP DRAWINGS SHOWING ALL DETAILS AND RICATION. SHOP DRAWINGS WILL NOT BE REVIEWED UNLESS SEALED BY A PROFESSIONAL ENGINEER TIONS AND COMPONENTS DESIGNED BY THE FABRICATOR. THIS ENGINEER OR HIS REP. SHALL VISIT THE	3. INSTALL DECKING CONTINUOUS OVER 3 SPANS EXCEPT WHERE OTHERWISE APPROVED.
CONSULTANT TO THIS EFFECT. THE ENGINEER SHALL ALSO PROVIDE SEALED SKETCHES FOR ALL FIELD	4. WELD DECK TO STEEL WITH 20 mm (3/4") FUSION WELDS IN PATTERN 36/11 & @ 150 MM (6") c/c ALONG ALL PERIMETER BEAMS AND JOISTS. LONGITUDINAL WELDS TO BE 200 mm (8") MAX. AND CLINCH SIDE LAPS TO BE 150 mm (6") MAX.
3 WITH THE FOLLOWING GRADES; 350 W	5. WELDERS SHALL BE CERTIFIED BY THE CANADIAN WELDERS BUREAU TO THE REQUIREMENTS OF CSA W47.1-09, DIV. 1 OR 2.1. THE INSTALLATION WELDING PROCEDURES SHALL BE APPROVED BY THE CANADIAN WELDING BUREAU. THE WELDERS SHALL HAVE CURRENT QUALIFICATIONS FOR ACT SPOT WELDING BY THE C.W.B.
350 W 350 W 300 W	6. SUBMIT TWO SETS OF PRINTS OF SHOP DRAWINGS SHOWING DETAILS, MATERIAL SPECIFICATIONS, FASTENING SCHEDULE AND DESIGN LOADS.
300 W ASTM A53 GR.B	7. WATER ON THE STEEL DECK OR BETWEEN THE DECK AND THE SUPPORTING STEEL SHALL BE REMOVED PRIOR TO PUDDLE WELDING OR STUD WELDING.
IIMUM M20. DESIGN BOLTED CONNECTIONS TO ASTM A325 ASSUMING THREADS IN THE SHEAR PLANE.	8. CHECK MECHANICAL AND ARCHITECTURAL DRAWINGS FOR OPENINGS, MECH. UNITS, HOLES, ETC. TO BE MADE IN METAL
TIFIED TO CSA W47.1-09 WELDING OF REINFORCING BARS SHALL CONFORM TO CSA W186-M1990(R2012). E 5mm FILLET WELDS AND WHERE EXPOSED IN FINISHED BUILDING WELDS SHALL BE GROUND SMOOTH. CHORS SHALL BE FUSION WELDED TO PLATES AS PER MANUFACTURERS SPECIFICATIONS AND	DECKING. OPENINGS SHOWN ON STRUCTURAL DRAWINGS FOR MECHANICAL UNITS, DUCTS AND PIPES ARE APPROXIMATE ONLY IN SIZE, LOCATION AND QUANTITY. THE MECHANICAL CONTRACTOR MUST PROVIDE CONFIRMATION OF THE LATEST INFORMATION TO THE GENERAL CONTRACTOR AT THE STRUCTURAL STEEL AND METAL DECK SHOP DRAWING APPROVAL STAGE OF THE PROJECT. CUT OPENINGS IN DECKING WHERE INDICATED ON THE DRAWINGS AND IN COOPERATION WITH OTHER TRADES. REINFORCE OPENINGS BETWEEN 150 AND 450 WITH 75 X 75 X 6 ANGLE PERPENDICULAR TO FLUTES. OPENINGS LARGER THAN 450mm FRAMED BY STRUCTURAL ERECTOR.
FURAL DRAWINGS SHALL BE DESIGNED BY THE STEEL FABRICATOR. MINIMUM BEAM SHEAR IS 70% OF IN "CISC MANUAL BEAM LOADS TABLES" FOR THE GIVEN SPAN OF THE BEAM U.N.O. UNUSUAL LOADINGS LEGEND FOR EXPLANATION OF THESE LOADS.	 PERIMETER DECK CLOSURES TO BE PROVIDED BY THE STEEL DECK CONTRACTOR UNLESS STRUCTURAL STEEL DECK CLOSURES ARE SHOWN ON THE DRAWINGS. CLOSURES SHALL PROVIDE STRAIGHT AND TRUE FINISHED EDGE (+/-0 6 mm). PROVIDE SUPPORT FOR METAL DECK AT WALLS AND OPENINGS WHERE REQUIRED.
NS SHALL BE SIMPLE CONNECTIONS. FOR BEAMS TO FACE OF HSS COLUMN CONNECTIONS PROVIDE VER CISC HANDBOOK.	N. TROVIDE SCHORT FOR MEI'NE DECK NT WILLES NID OFENINGS WILLRE REQUIRED.
ER EISE MAADDOOR. DR DETAILED ON THE STRUCTURAL DRAWINGS (I.E. SMALL FRAMING MEMBERS) USE ANY TYPE OF ECIFIED LOAD SHOWN.	LOAD BEARING MASONRY WALL:
LL BE DESIGNED TO DELIVER SHEAR ONLY TO THE FACE OF THE COLUMN. SEISMIC AND DRAG STRUT	1. ALL LOAD BEARING HOLLOW BLOCK UNITS TO HAVE AN ULTIMATE COMPRESSIVE STRENGTH 20 MPa (2900 psi.) MIN. UNLESS NOTED.
ER SHEAR ONLY TO THE CENTER LINE OF THE COLUMN. RE TO BE WELDED OR BOLTED WITH HIGH STRENGTH BOLTS IN BEARING TYPE CONNECTIONS (MIN. 2 - 20	2. MORTAR FOR LOAD BEARING MASONRY TO BE TYPE "S" OR BETTER WITH A MIN. AVERAGE COMPRESSIVE STRENGTH OF 13 Mpa (1900 psi.) @ 28 DAYS MIN.
ES ARE TO BE PRE-TENSIONED. THE PRIME STRUCTURAL CONSULTANT SHALL HAVE FINAL APPROVAL ON	 ALL AVAILABLE BEARING AREAS OF MASONRY UNITS SHALL BE FULLY COVERED WITH MORTAR SPREAD IN AN EVEN LAYER AND ALL VERTICAL JOINTS INCLUDING JOINT BETWEEN BRICK AND BLOCK) SHALL BE FILLED SOLIDLY WITH
N TO BE DESIGNED BY CONTRACTOR (WHOEVER IS RESPONSIBLE FOR ERECTION). CONTRACTOR IS	MORTAR. 4. FOR BONDING OF BRICK AND BLOCK USE HEAVY DUTY TRUSS-TYPE (OR EQUIVALENT ADJUSTABLE TYPE) 'BLOCK-LOC'
	 or EQUAL @ 400mm (16") c/c VERT. COMPLETELY EMBEDDED IN MORTAR. ALL INTERSECTION MASONRY WALL TO HAVE MASONRY BOND OR HEAVY-DUTY 'BLOC-LOC' OR EQUAL @ 400mm (16") c/c
AND ELECTRICAL DRAWINGS TO AVOID CONFLICT WITH STRUCTURAL ELEMENTS. N STRUCTURAL STEEL ANYWHERE.	VERTICALLY.
S AND NON-STRUCTURAL ARCH. STEEL NOT DETAILED ON STRUCTURAL DRAWINGS, STRUCTURAL D COMMENT ON THE ABILITY OF THE SHOWN MEMBERS AND CONNECTIONS TO RESIST LOADS AND	6. PROVIDE SOLID OR FULLY GROUTED MASONRY PAD UNDER ALL JOISTS AND BEAMS. SEE PLANS AND SCHEDULES FOR BEARING DETAILS.
D COMMENT ON THE ABILITY OF THE SHOWN MEMBERS AND CONNECTIONS TO RESIST LOADS AND DING CODE 2006. OVERALL DETAILING TO COMPLY WITH ARCHITECTURAL DRAWINGS AND ARE THE LINGS AND STAIR SHOP DRAWINGS TO BE STAMPED BY P.ENG. OF ONTARIO.	 PROVIDE 'BLOC-LOC' MASONRY WALL REINFORCING OR EQUAL FOR ALL MASONRY WALLS, INSTALLED ACCORDING TO THE MANUFACTURER'S INSTRUCTIONS, AT 400 mm (16") c/c VERT. MAXIMUM.
CES EXPOSED TO WEATHER AND NOT REQUIRING FIREPROOFING SHALL BE PAINTED WITH ONE COAT OF TECTURAL DRAWINGS). ALL SITE WELD AND WELDING CONNECTIONS TO BE TOUCHED-UP WITH	8. REFER TO ARCH. DWGS. FOR LOCATIONS OF CONTROL JOINTS IN MASONRY WALLS.
	9. ALL MASONRY PIERS TO BE SOLID OR FULLY GROUTED MASONRY.
CAST INTO CONC. TO FACILITATE NAILING TO FORM WORK. RAMING AT OPENINGS.	11. ALL MASONRY WALLS TO HAVE A MINIMUM OF 15M @ 24" o.c. VERTICAL REINFORCING STEEL, AT WALL ENDS, CORE BESIDE WINDOW DOOR OR OTHER OPENINGS, AND AT ALL WALL INTERSECTIONS.
RAWING AT OFENINGS. RAWINGS FOR OPENINGS, MECHANICAL UNITS, HOLES, ETC. TO BE MADE. OPENINGS SHOWN ON NITS, DUCTS AND PIPES ARE APPROXIMATE ONLY IN SIZE AND LOCATION. THE MECHANICAL	12. PROVIDE BLOCK WALL CONTROL JOINT AT LOCATION SHOWN ON ARCHITECTS DRAWINGS. MAXIMUM SPACING TO BE AT 15.2 METERS (50'-0"). REINFORCE ON CELL ON EITHER SIDE OF JOINT WITH 2 - 15M VERTICAL GROUT SOLID.
NTIS, DUCTS AND PIPES ARE APPROXIMATE ONLY IN SIZE AND LOCATION. THE MECHANICAL OF THE LATEST INFORMATION TO THE GENERAL CONTRACTOR AT THE STRUCTURAL STEEL AND METAL HE PROJECT.	13. USE RUNNING BOND BLOCK CONSTRUCTION. KEY ALL MASONRY JOINTS AT WALL CORNERS AND INTERSECTIONS. RAKE BACK WALL CONSTRUCTION WHEN TURNING WALL CORNERS.
RTING STEEL FRAME AND IS DEPENDENT UPON DIAPHRAGM ACTION THE DECK FLOORS AND BILITY AND FOR RESISTANCE TO WIND AND SEISMIC FORCES. PROVIDE ALL TEMPORARY SUPPORTS	14. REINFORCE TWO CORES UNDER STEEL BEAM BEARING WITH 2-15M VERTICAL EACH CORE FULL HEIGHT AND GROUT SOLID. PROVIDE 2 SOLID COURSES UNDER LINTEL BEARING. NO DUCT OR VOIDS ALLOWED AT BEARING.
CE TO WIND AND SEISMIC FORCES UNTIL THESE ELEMENTS ARE COMPLETE AND ARE CAPABLE OF	15. LINTEL - INSTALL STEEL LINTEL. PROVIDE BLOCK LINTELS ONLY IN SPECIFIC AREAS SPECIFIED IN STRUCTURAL DRAWINGS. BLOCK LINTEL TO BE TWO COURSE (400mm) DEEP COMPLETE WITH 4-15M TOP AND BOTTOM FOR OPENINGS 2000mm OR LESS. EXTEND REINFORCING 650mm (24") PAST OPENINGS BOTH SIDES.
ATION SHIP OF MEMBERS CONNECTED. CONNECTION DETAILS INDICATED ON THE DRAWINGS SHALL BE TION DESIGN. SEE SPECIFICATIONS. ALL SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY THE R'S SEAL FOR THE PROVINCE WHERE THE STRUCTURE IS LOCATED. ENGINEER'S SEAL MAY BE QUALIFIED	 VERTICAL CORE FILLS TO BE CAST IN LIFTS OF 1200mm (4'-0") MAXIMUM. VERTICAL REINFORCING TO HAVE A MAXIMUM LENGTH OF 2000mm (6'-8") WITHOUT SPLICING. ALL LAPS AND ANCHORAGES IN REINFORCING TO BE 40mm DIAMETER MINIMUM UNLESS NOTED OTHERWISE.
ING WALL LINTELS NOT SHOWN ON STRUCTURAL DWG. REFER TO ARCH. & MECH. DWG. FOR OPENING	17. TEMPORARY BRACING AND SHORING TO BE PROVIDED DURING CONSTRUCTION. BRACING AND SHORING DESIGN IS THE CONTRACTORS RESPONSIBILITY.
CHMENT OF OTHER MATERIALS EXCEPT AT CRITICAL TENSILE SECTIONS OF BEAMS.	OPEN WEB STEEL JOIST
LOWED WITHOUT THE ENGINEER'S APPROVAL. 100% BUTT WELDS IN SPLICES ARE TO BE CEPTED BY A WELDING INSPECTION COMPANY.	 DESIGN AND FABRICATE OPEN WEB STEEL JOISTS TO CAN/CSA S16-14 (S16 AND CSA S136-07(R2012) FOR COLD ROLLED SECTIONS) FOR DEPTHS AND LOADING SHOWN ON THE DRAWINGS. DESIGN JOISTS FOR A FACTORED POINT LOAD OF 2.0
nm (8") BEARING UNLESS OTHERWISE NOTED.	KN APPLIED AT ANY PANEL POINT.CAMBER REQUIREMENTS AND DEFLECTION LIMITATIONS TO CSA S16-14 UNLESS NOTED ON DRAWINGS.
ICE WITH THE O.B.C. AT ALL LOCATIONS WHERE COLUMNS, BEAMS OR JOISTS IN CONTACT WITH OR	3. JOIST SEAT DESIGN REQUIREMENTS:
HAVE 200mm (8") MIN. BEARING UNLESS NOTED AND TO BE ANCHORED TO MASONRY WITH 38X6 (1 ½" X ED TO BEAM WEB AT TOP OF BEAM.	A) TOTAL JOISTS SEAT DEPTH TO BE 100mm UNLESS OTHERWISE NOTED. B) ALL JOIST SEAT CONNECTIONS TO BE WELDED TO JOIST P.ENG. DESIGN.
	4. MINIMUM GAUGE OF TOP CHORD TO BE 3.2mm.
	5. EXTEND BOTTOM CHORD WHERE INDICATED ON FRAMING PLAN. DESIGN CHORD EXTENSIONS FOR 10 KN FACTORED TENSION OR COMPRESSION, UNLESS NOTED OTHERWISE.
	6. JOISTS BEARING ON MASONRY WALLS TO HAVE 100mm (4") MINIMUM BEARING UNLESS NOTED.
	 MINIMUM BEARING OF JOISTS ON STEEL SHALL BE 65MM (2-½"). PROVIDE TIE JOISTS (T.J.) AT ALL COLUMNS.
	 9. DELIVER ALL MATERIALS TO JOB SITE WHEN REQUIRED BY THE STRUCTURAL STEEL CONTRACTOR. IN CONFORMANCE WITH THE GENERAL CONSTRUCTION SCHEDULE.
	 WITH THE GENERAL CONSTRUCTION SCHEDULE. 10. ALL VISUALLY EXPOSED SURFACES OR SURFACES EXPOSED TO WEATHER AND NOT REQUIRING FIREPROOFING SHALL BE PAINTED WITH ONE COAT OF PRIMER TO CISC/CPM 1-73A.
	11. OPEN WEB STEEL JOIST SUPPLIER TO PROVIDE RECOMMENDED ERECTION PROCEDURE INSTRUCTIONS TO THE SITE
	SUPERINTENDENT AS PER WCB REGULATION 34.42 "ERECTION INSTRUCTIONS". 12. OPEN WEB STEEL JOIST SUPPLIER TO COORDINATE WITH MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS TO AVOID CONFLICTS WITH STRUCTURAL ELEMENTS.
	AVOID CONFLICTS WITH STRUCTURAL ELEMENTS. 13. NO MECHANICAL EQUIPMENT TO BE INSTALLED ON ROOF UNLESS CALLED FOR ON STRUCTURAL DRAWINGS.
	14. ALL JOISTS ARE TO BE DESIGNED TO WITHSTAND LIVE + DEAD LOAD, POINT LOADS AND MOMENTS SHOWN ON PLAN AND
	ALL WIND UP-LIFTS IF APPLICABLE AS CALLED FOR IN O.B.C. LATEST ISSUE. 15. JOISTS SHALL BE PROVIDED COMPLETE WITH BRIDGING, BEARINGS ANCHORAGE, CONNECTION DETAILS, CAMBER, ETC., AS REQUIRED. ALL JOIST SHOP DRAWINGS, DESIGN CALCULATION AND COMPUTER OUTPUTS SHALL BEAR THE SIGNED
	 AS REQUIRED. ALL JOIST SHOP DRAWINGS, DESIGN CALCULATION AND COMPOTER OUTPUTS SHALL BEAR THE SIGNED SEAL OF A PROFESSIONAL ENGINEER OF ONTARIO TO BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO WORK. 16. NO COLD FORMED ANGLES OR SHAPES LESS THAN 5mm (³/₁₆") THICK WILL BE ACCEPTED.
	17. NO LOADS ARE TO BE APPLIED TO JOIST CHORDS BETWEEN PANEL POINTS OTHER THAN THOSE INCLUDED IN JOIST DESIGN.

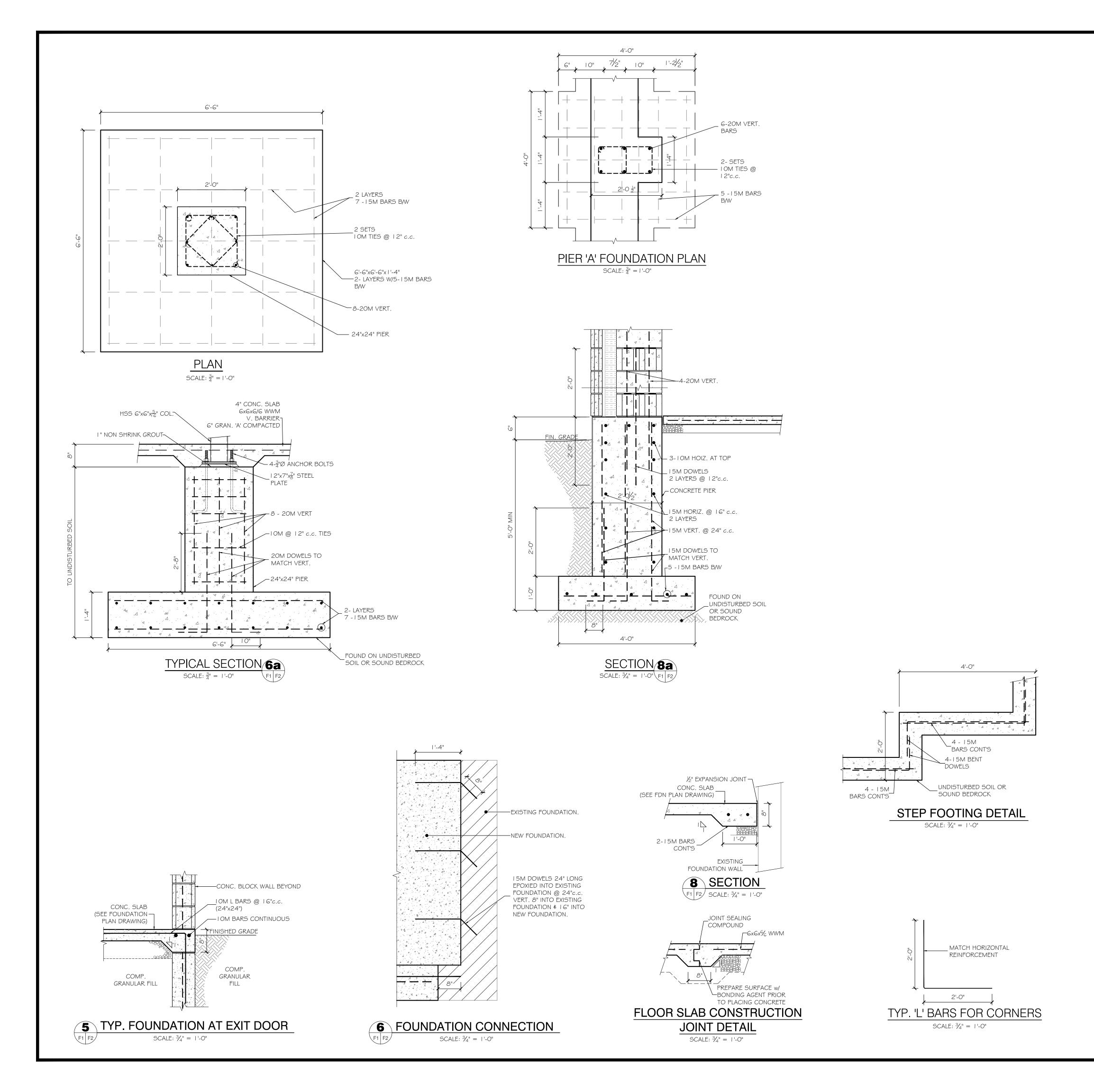
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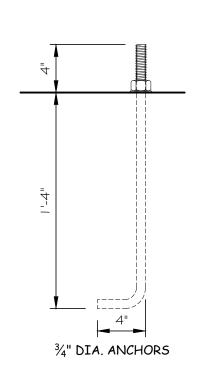
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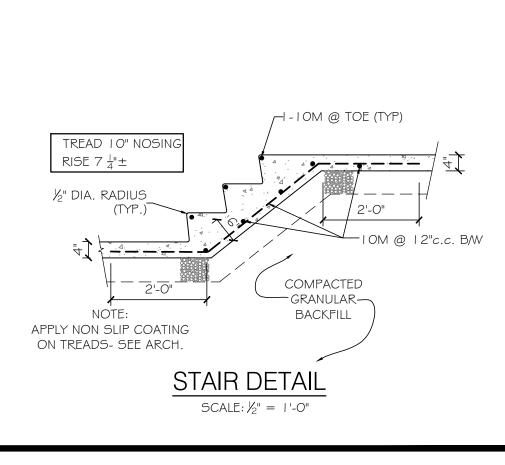
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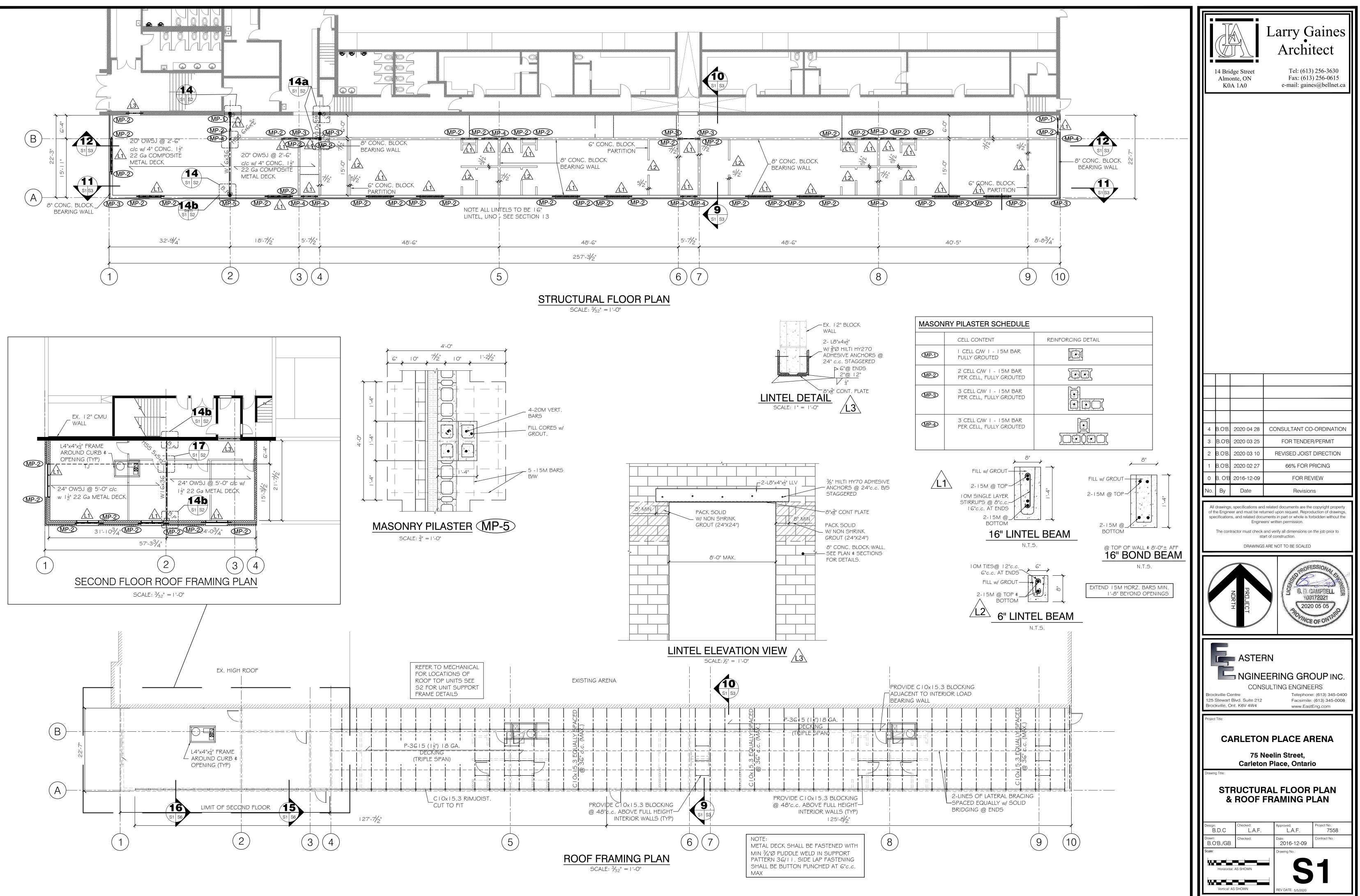


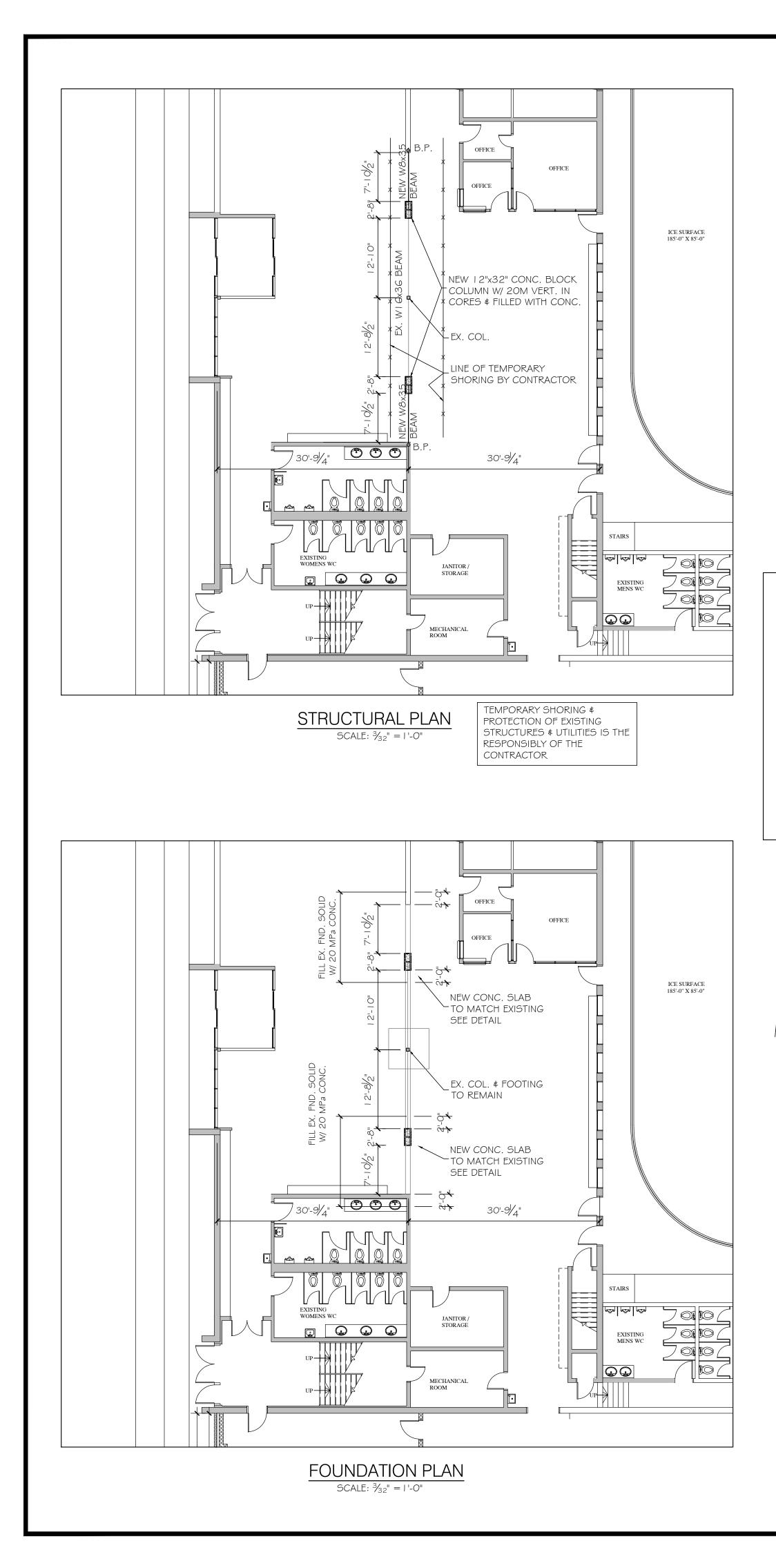
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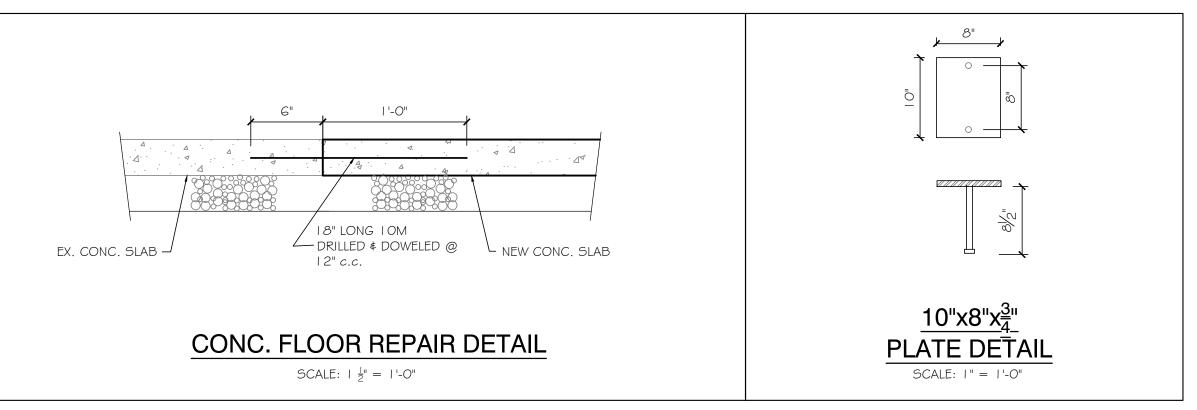












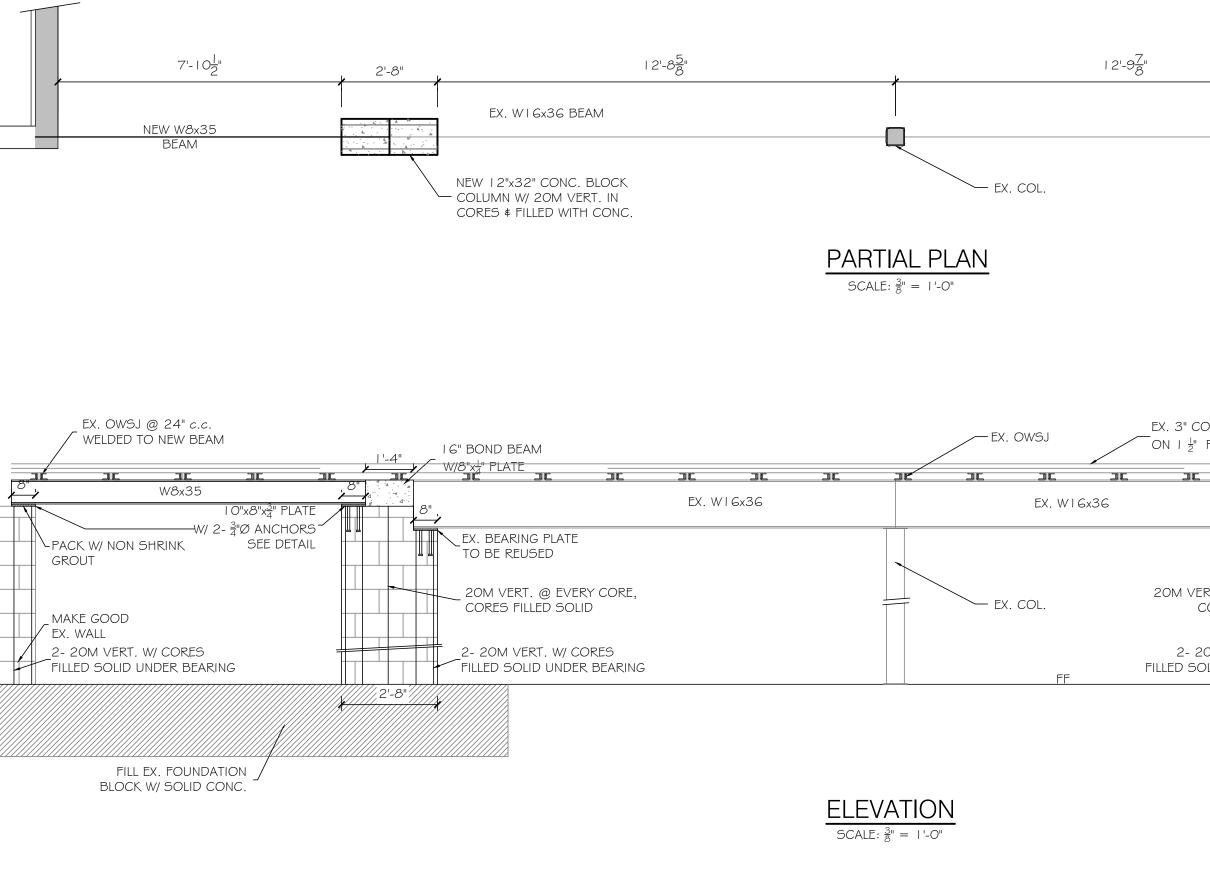
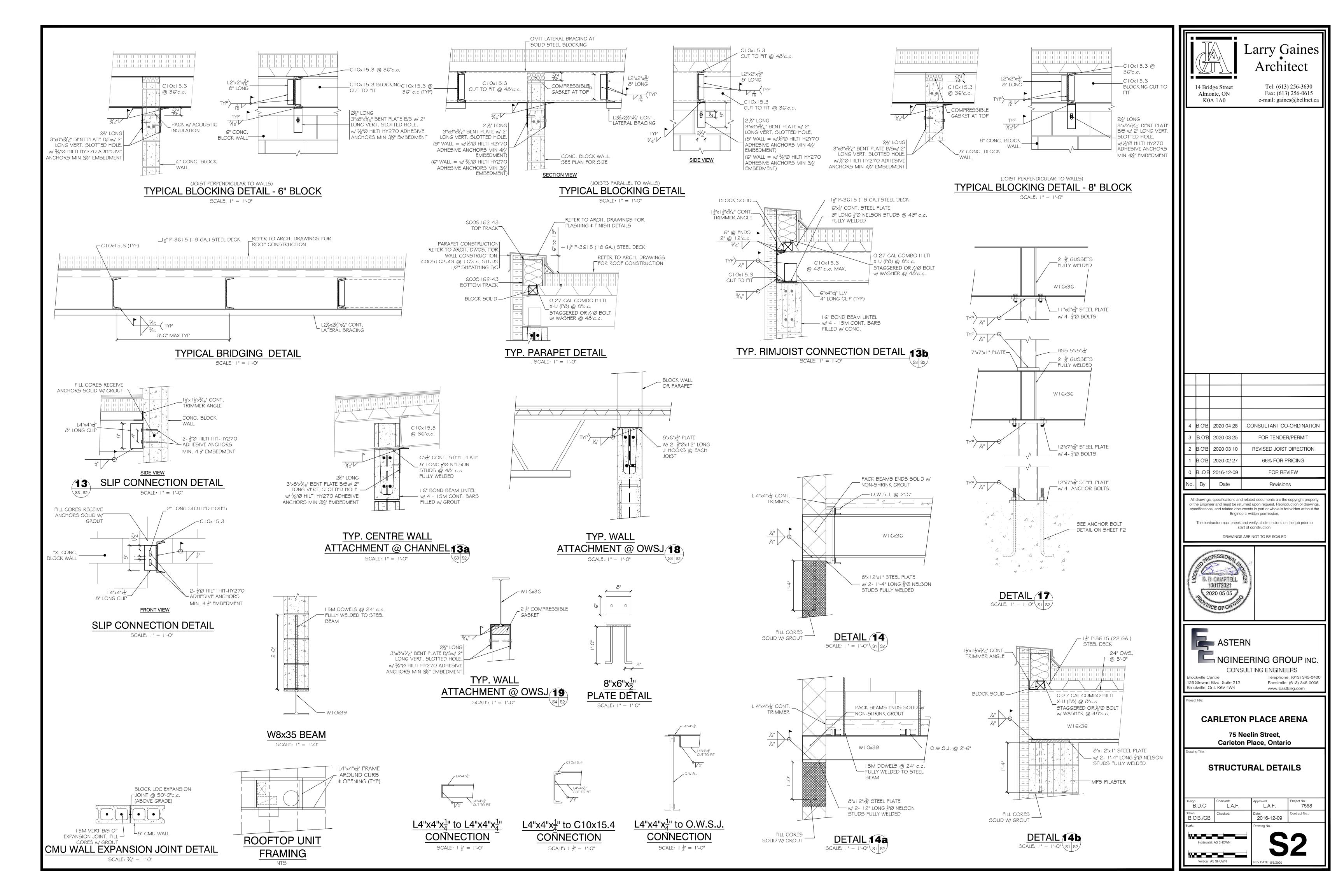
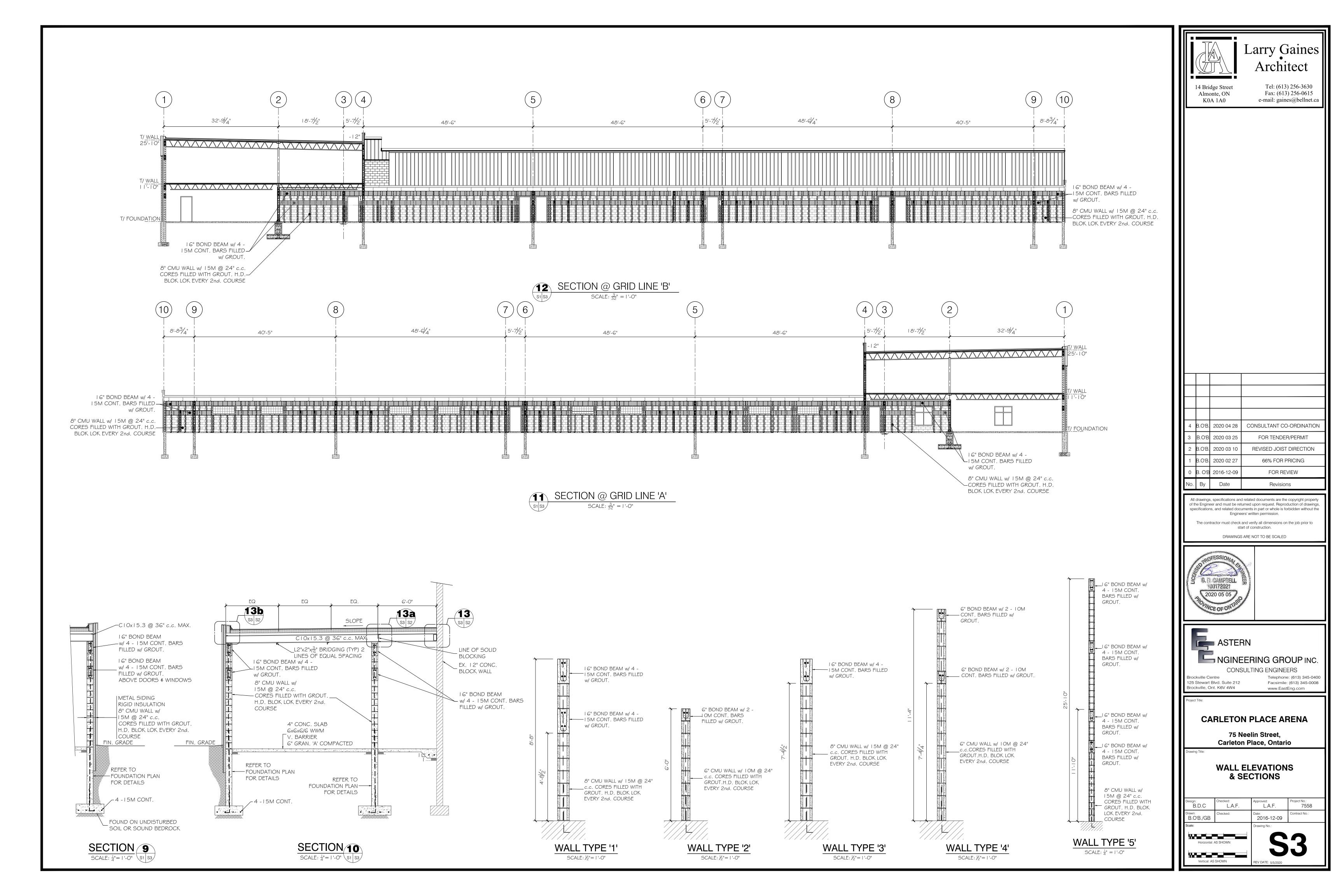
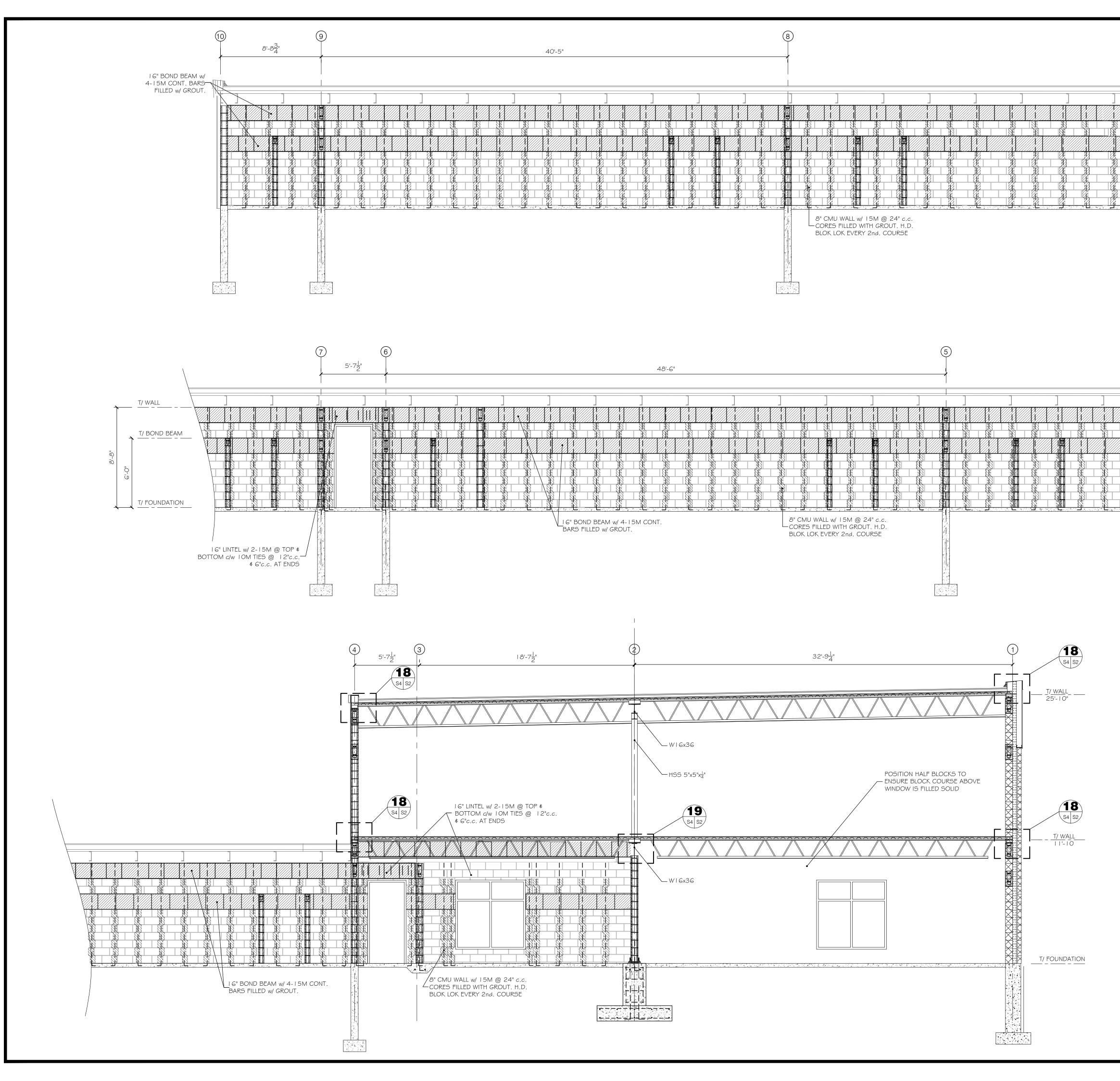


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